



Prepared for;

Jennings O'Donovan

Inchamore Windfarm (IWF)

Site Investigation Report & Peat & Subsoil Stability Risk Assessment



603679 IWF EIAR App. 8.1 SI & PSSRA (02)





MARCH 2023



RSK GENERAL NOTES

Project No.: 603679 (02)
Title: Site Investigation & Peat & Subsoil Stability Risk Assessment Report
Client: Jennings O'Donovan
Date: 03/04/2023
Office: RSK Dublin
Status: (03) FINAL

Author	<u>Lissa Colleen McClung</u>	Technical reviewer	<u>Sven Klinkenbergh</u>
Signature		Signature	
Date:	<u>03/04/2023</u>	Date:	<u>26/04/2023</u>

RSK (Ireland) Ltd (RSK) has prepared this report for the sole use of the client, showing reasonable skill and care, for the intended purposes as stated in the agreement under which this work was completed. The report may not be relied upon by any other party without the express agreement of the client and RSK. No other warranty, expressed or implied, is made as to the professional advice included in this report.

Where any data supplied by the client or from other sources have been used, it has been assumed that the information is correct. No responsibility can be accepted by RSK for inaccuracies in the data supplied by any other party. The conclusions and recommendations in this report assumes that all relevant information has been supplied by those bodies from whom it was requested.

No part of this report may be copied or duplicated without the express permission of RSK and the party for whom it was prepared.

Contents

1. Introduction	5
1.1 Background	5
1.2 Purpose	5
1.3 Scope of Works – Tender	5
1.4 Statement of Authority	5
2. Site Investigation Works & Methods	6
2.1 Scope of Works – Completed	6
2.2 Peat & Slope Stability Risk Assessment Methodology	6
2.3 Subsoil & Slope Stability Risk Assessment Methodology	12
3. Baseline Conditions	15
3.1 Site Description & History	15
3.2 Site Geology	15
3.3 Site Soils & Subsoils	15
3.4 Topography & Substrate Topology	15
3.5 Hydrology & Climate	16
3.6 Receptors	16
4. Site Investigation Data & Results	17
4.1 Peat Depth Data	17
4.2 Trial Pit Data	17
4.3 Borehole Data	17
4.4 Peat Stability Risk Assessment Results	18
4.5 Peat Stability Risk Assessment Interpretation	25
4.6 Subsoil Stability Risk Assessment Results	27
4.7 Subsoil Stability Risk Assessment Interpretation	28
5. Conclusions	30
6. Caveats & Recommendations	32
7. References	33

List of Figures

Figure 1: Correlation Between Moisture Content and Shear Strength of Peat (N. Boylan, P. Jennings & M. Long, 2008)	10
Figure 2: Correlation Between Factor of Safety, Cohesive Strength and Depth of Subsoils	28

List of Appendices

SI Appendix	Title
SI Appendix A	Peat Depth Probing Locations
SI Appendix B	Peat Depth & Subsoil Databases
SI Appendix C	Trial Pit & Borehole Locations
SI Appendix D	Trial Pit Logs
SI Appendix E	Trial Pit and Site Photos
SI Appendix F	Borehole Logs & Bedrock Core Testing Laboratory Certificates
SI Appendix G	Subsoil Testing - Laboratory Certificates
SI Appendix H	Register of Geo-Hazards
SI Appendix I	Peat and Subsoil Stability Risk Assessment

1. Introduction

1.1 Background

RSK Ireland was commissioned by Jennings O'Donovan & Partners (JOD, the Client) on behalf of Inchamore Wind DAC (the Developer/s) to assess the geological site characteristics in relation to the planning application for the Inchamore Wind Farm (IWF, the Development) in Co. Cork.

1.2 Purpose

Site Investigation for the purposes of assessing ground conditions at EIA design phase of a proposed wind farm development, Inchamore Wind Farm, Co. Cork. Assessing ground conditions in terms of peat and slope stability risk, subsoil and geological characterisation and classification.

1.3 Scope of Works – Tender

The scope of works was initially specified by the Developer at tender phase. The scope of works for ground investigations at tender included the following works;

- Peat probing (50 m grid), 50 ha
- Trial pits, 35 no.
- Number of groundwater monitoring wells, 4 no.
- SI report with detailed findings, records and interpretation

Provisional works included;

- Gouge auger samples
- Boreholes up to 15 m, 5 no.
- Ground penetrating radar surveys (5 days)

In consultation with the Client and Developer the scope of works was adapted to the site based on observations made by desk study and initial site walk overs and assessments. The actual completed scope of works is detailed in **Section 2**.

This work has been carried out in unison with the EIAR for the Project. Therefore, this report will be appended to **EIAR Chapter 8 - Soils & Geology** as part of the planning application for the Project. The EIAR tender scope includes for a stand-alone Peat Stability Report as well as stand alone Site Investigation report, however the two will be merged in this Site Investigation report. This is done with a view streamlining the site geological assessment.

Further to the above, the geological or environmental setting of the site will be described in detail in **EIAR Chapter 8 – Soil & Geology** with appended maps and graphics for reference. This report will refer and summarise the EIAR chapter/s to avoid duplication of information or graphics. This report will also reference **EIAR Chapter 9 – Hydrology & Hydrogeology** in relation to groundwater.

1.4 Statement of Authority

RSK (Ireland) Ltd. (RSK), part of RSK Group, is a consultancy providing environmental services in the hydrological, hydrogeological and other environmental disciplines. The company and group provide consultancy to clients in both the public & private sectors. More information can be found at www.rskgroup.com. The principal members of the RSK EIA team involved in this assessment include the following persons;

- Sven Klinkenbergh – B.Sc. (Environmental Science), P.G.Dip. (Environmental Protection) – Associate, Project Manager and EIA Lead Author with c. 10 years industry experience in the preparation of hydrological, hydrogeological and geological reports..
- Project Scientist: Lissa Colleen McClung - B.Sc. (Hons.) Environmental Studies, M.Sc. (Hons.) Environmental Science. Current Role: Graduate Project Scientist
- Project Scientist: Mairéad Duffy – B.Sc. (Environmental Science), M.Sc. (Climate Change). Current Role: Graduate Project Scientist

2. Site Investigation Works & Methods

2.1 Scope of Works – Completed

The completed scope of works included;

- Peat depth probing, approx. 150 no. sampling locations.
- Trial pits, 16 no.
- Sub-soil sampling and Particle Size Distribution analysis, 4 no.
- Drilling – Rotary Core, 1 no.
- Drill core sample analysis. Point Load (PL) and Unconfined Compression Test (UCS).

2.2 Peat & Slope Stability Risk Assessment Methodology

2.2.1 Key assessment principals

The site assessment is carried out following key principals in line with relevant guidance, namely;

- BS 5930:2015+A1:2020 Code of Practice for Site Investigations.
- Scottish Government (2017) Peat Landslide Hazard and Risk Assessments: Best Practice Guide for Proposed Electricity Generation Developments

Some key insights to application and interpretation are provided from numerous documents, in particular;

- N. Boylan, P. Jennings & M. Long (2008) Peat slope failure in Ireland. Quarterly Journal of Engineering Geology and Hydrogeology.

2.2.1.1 BS 5930 – Code of Practice for Site Investigations

This document explains the important steps to be taken in preparing for, scoping, and executing site investigations of various nature. The standard covers the following aspects:

- Planning: This section provides guidance on the planning of site investigations, including the purpose of the investigation, the scope of work, and the selection of appropriate investigation techniques.
- Desk Study: This section provides guidance on the collection and review of existing information, such as geological maps, site records, and historical data, that can aid in the planning and execution of site investigations.
- Site reconnaissance: This section provides guidance on the preliminary site visit to collect data on site characteristics and conditions.
- Investigation methods: This section provides guidance on the selection of appropriate investigation methods, such as drilling, sampling, and testing techniques, based on the site characteristics and the purpose of the investigation.
- Field testing: This section provides guidance on the execution of field testing, such as in-situ testing, geophysical surveys, and environmental testing.
- Laboratory testing: This section provides guidance on the selection and execution of laboratory testing, such as soil and rock testing, and the interpretation of laboratory results.
- Reporting: This section provides guidance on the reporting of site investigations, including the presentation of data, the interpretation of results, and the conclusions and recommendations.

Scoping site investigations and sampling regime in terms of sampling locations and frequency is an important and dynamic process. While BS 5930 details sampling frequency in terms of soil and rock geotechnical and environmental testing, standard provides guidance on the spacing and frequency of sampling points, which may vary depending on the site conditions, the purpose of the investigation, and the type of sampling method being used. It is important to scope and align appropriate methodologies and sampling regime with specific objectives and within specific environments, including Peat & Slope Stability Risk Assessments in peatland areas.

2.2.1.2 Scottish Government (2017) Peat Landslide Hazard and Risk Assessments: Best Practice Guide for Proposed Electricity Generation Developments

The Scottish Government's Peat Landslide Hazard and Risk Assessments: Best Practice Guide for Proposed Electricity Generation Developments is a document that provides guidance on the assessment of landslide

hazard and risk in peatland areas, particularly in relation to proposed electricity generation developments. The document is published and written in context of Scottish peatlands, however in the absence of relevant guidance, it is widely accepted as relevant guidance in Ireland.

The guide emphasizes the need for a comprehensive assessment of landslide hazard and risk in peatland areas, which is particularly important due to the unique characteristics of these environments. Peatlands are often found in areas of high rainfall, and the accumulation of peat can result in unstable ground conditions, which can increase the risk of landslides.

The guide provides a step-by-step approach to landslide hazard and risk assessment, including the identification of potential landslide triggers, the characterization of the peatland environment, the assessment of landslide susceptibility, and the estimation of landslide hazard and risk. The guide also provides guidance on the selection of appropriate methods for landslide hazard and risk assessment, such as field mapping, remote sensing, and numerical modelling. The guide emphasizes the importance of stakeholder engagement and communication in the landslide hazard and risk assessment process, particularly in relation to proposed electricity generation developments, which can potentially have significant impacts on the surrounding environmental receptors and communities. The guide covers the following aspects which should be included in the site risk assessment;

- **Sampling Regime:** The guide recommends a sampling regime that includes both surface and subsurface surveys, using techniques such as; depth probing, gouge coring, trialpitting, drilling, and geophysical surveys. The aim is to obtain a comprehensive understanding of the geology and hydrogeology of the site, as well as the depth and condition of the peat layer.
- **Assessment of Desk Top Data:** The guide recommends an assessment of desktop data to identify potential sources of instability, such as steep slopes, drainage features, and areas of peat degradation. This assessment should be based on available data sources such as geological maps, aerial photographs, and LiDAR data.
- **Degree of Geomorphological Assessment:** The guide recommends a high degree of geomorphological assessment, using methods such as aerial photography interpretation and field mapping to identify potential instability features such as landslides and erosion channels. Many sources of data can input to the interpretation of stability risk at any particular location, and field reconnaissance is also a valuable tool in this respect.
- **Interpretation of Data:** The guide recommends a detailed interpretation of all data collected, including the results of field surveys and laboratory testing. This should involve the identification of key parameters such as peat depth, soil properties, and groundwater levels or saturation, as well as the integration of all available data to develop a comprehensive understanding of the potential for instability. This can result in screening out peat stability risk, for example; in areas of extensive shallow bedrock or bedrock outcrops, or areas with very minor inclines. Conversely, high risk areas can potentially be identified by desk top assessment alone, for example; steep slopes in excess 15 degrees, or areas with historical stability issues or historic landslides.
- **The development of numerical models for peat stability risk assessments** has been driven by advances in computer technology (e.g. QGIS) and modeling techniques, as well as an increased awareness of the risks associated with peat instability. The use of numerical modeling in peat stability risk assessments typically involves the following steps:
 - **Development of a conceptual model:** This involves the development of a conceptual model of the site based on the results of field investigations and laboratory testing. The conceptual model should include information on the geometry and properties of the peat layer, as well hydrogeological characteristics such as pore water pressure or bul unit weight (saturation).
 - **Selection of appropriate modeling techniques:** There are a variety of modeling techniques that can be used to simulate peat stability, including finite element and finite difference methods. The selection of an appropriate modeling technique will depend on the specific characteristics of the site and the goals of the assessment.
 - **Calibration and validation of the model:** The model is calibrated and validated using data collected during field investigations and laboratory testing. This involves adjusting model parameters to improve the match between simulated and observed data.

Overall, the guide emphasizes the importance of a comprehensive and integrated approach to peat landslide hazard and risk assessments, which includes a thorough sampling regime, an assessment of desktop data, a high degree of geomorphological assessment, and a detailed interpretation of all data collected. By following these guidelines potential hazards and risks associated with peat instability can be identified and managed effectively.

2.2.2 Desktop baseline characterisation & approach

The site and proposed development are assessed using QGIS mapping software with relevant environmental data layers published by relevant bodies including; EPA, and GSI.

Open source Global Digital Elevation Model (DGEM) data is used to determine the general nature of the topography at the site, including interrogating elevation data to determine slope inclines across the site.

Areas of the site undergo preliminary risk assessment and development constraints are identified and mapped. This will include slope inclines >8 degrees, 50m and 150m surface water or other environmental receptor buffers, etc. This data is used to inform the initial design phase of a project and to scope the site survey and sampling regime.

On completion of the initial phases of site surveys, georeferenced data is compiled and mapped in QGIS along with the initial desktop data. The site undergoes further preliminary risk assessment, preliminary modelling and constraints are updated and the process repeats i.e. phase 2.

Other environmental data, including peatland ecological data is incorporated where relevant.

2.2.3 Peat depth probing & topography assessments

Peat depth probing was undertaken at the site including at each proposed potential turbine location, at proposed locations for other infrastructure, and elsewhere on site where desktop assessment could not screen out stability risk.

Depth probing was conducted using a fibreglass depth probe and at each survey point the depth of peat, local incline (incline within a c. 5-10 m radius of the survey point) and grid reference (Irish Grid) were recorded. Notes on observations were also recorded including time of taking photographs, presence of drains etc.

A number of inferred peat depth probe points with a value of 0.5m, distributed in 2 no. transects at proposed turbine location T2. The inferred transects are intended to assess variability of peat stability corresponding with variability of incline, and to risk assess stability in close proximity to sensitive receptors.

2.2.4 Peat gouge coring & qualitative assessments

Gouge coring of peat was carried out to a limited extent (peat depth generally shallow). Peat quality assessment were made at existing cuttings and during trial pitting.

2.2.5 Piezometer installation & groundwater assessments

Not applicable. Peat depth at the site observed to be shallow generally at the site.

2.2.6 Topography & substrate topology

Using available topographical data provided for the site and peat thickness / depth data obtained during MEL surveys, the topology (characteristics of a surface) of the substrate underlying the peat on site was assessed and cross sections generated to evaluate variance from the surface topology.

2.2.7 Peat stability numerical assessment

This stability assessment has been undertaken using a relatively simple infinite slope stability approach (Boylan, N, and Long, M, 2012) (derived from Bromhead's formula (Scottish Gov., 2017)), as follows;

$$FoS = \frac{c_u}{yz \sin \alpha \cos \alpha}$$

For the purpose of this assessment, the above formula will be referred to as the *FoS Formula*.

Qualifying peat stability at all peat survey points and trial pit locations was done using the following parameters;

Table 1: Formula Parameters & Symbols

Symbol	Description	Unit
FoS	Factor of Safety	FoS
c_u	Effective cohesion or Undrained Shear Strength	kPa
y	Bulk Unit Weight of Peat	kN/m ³
z	Depth to failure plain	m
α	Slope Angle	Degrees

The Factor of Safety (FoS) result will range from 0 to infinity, however the following ranges are prescribed ratings as follows;

Table 2: Factor of Safety (FoS) Classifications (Scottish Gov., 2017)

Description	FoS Value Range	Classification
Stable	>1.3	Acceptable
Marginally Stable	1.0 > < 1.3	Acceptable
Unstable	<1.0	Unacceptable

As per the guidance listed in Section 2 of this report, FoS values of 1.0 or greater are considered acceptable in terms of peat stability (Scottish Gov., 2017).

The assessment has been completed on the basis of 2 no. scenarios, which are as follows;

1. Scenario A – Peat stability in terms of the receiving environment as is, that is using the depth of peat observed and recorded during site surveys.
2. Scenario B – Peat stability in terms of the in-situ peat with 1m fill (presumed peat) placed on top, that is using the depth of peat observed and recorded during site surveys plus 1 metre fill (depth + 1.0m). This is the assessment worst case scenario, and this will be used to assess stability at proposed infrastructure locations.

Undrained shear strength (effective cohesion) (c_u) has been derived by means of assessing moisture content results, which is; there is a correlation between peat moisture content and shear strength (effective cohesion). Shear vane testing has been carried out on the site however, shear vane test, or in situ barrel shear tests are not considered representative of shear strength characteristics of the peat being assessed in terms of stability assessment given numerous flaws with the test itself, namely; the shear vane test evaluates the shear strength where by the force is exerted in a vertical and cylindrical plane, which is not indicative of forces at play with respect slope stability or mass movement; and fibres and roots within the peat will effect the test itself, potentially exaggerating, or giving misleading data. The following graph presents conceptual shear strength values for peat (Boylan N, Jennings P & Long M., 2008).

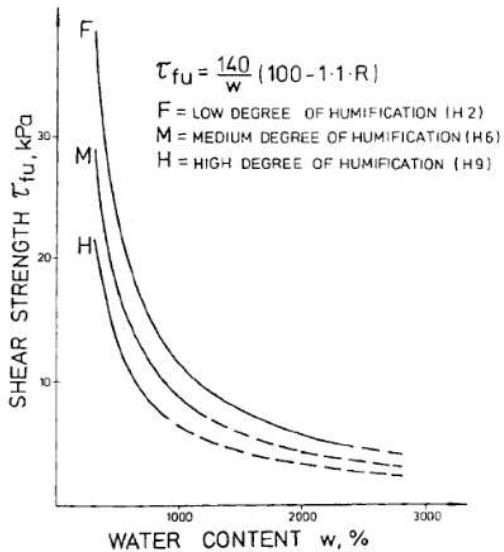


Figure 1: Correlation Between Moisture Content and Shear Strength of Peat (N. Boylan, P. Jennings & M. Long, 2008)

The following table presents the typical minimum, average and maximum moisture content which been used to determine indicative shear strength values for the Site.

Table 3: Peat Moisture Content Range & Indicative Shear Strength

Category	Moisture Content (%)	Indicative Shear Strength (kPa)
Minimum	200	>20
Average	750	10-20
Maximum	1500	<10

For the purpose of assessing peat stability for the Site a conservative undrained shear strength (effective cohesion) value will be used in numerical assessments, i.e., 3.5 kPa.

In situ bulk density (kg/m^3), or bulk unit weight (kN/m^3) of peat (γ) is typically within the range of 900-1100 kg/m^3 (Munro R, 2004), or 8.8-10.8 kN/m^3 . For the purpose of assessing peat stability for the Site a conservative bulk unit weight value will be used in numerical assessments i.e., 11 kN/m^3 .

The depth to failure plane (z) is presumed to be thickness or depth of peat at any given sampling point being assessed, however it should be noted that the failure plane can potentially be within peat (peat on peat movement), or the substrate i.e., weathered rock or underlying soils.

Slope angle (α) is presumed to be topographical incline measured on site / evaluated using high resolution elevation data at any given sampling point being assessed, however it should be noted that the slope angle (α) relates to the failure plane angle, which is presumed to be the peat and substrate interface, and which is presumed to be parallel to the surface when using FoS Formula (Infinite Slope Formula). In reality the underlying substrate is unlikely to be parallel to the surface topology.

It should be noted that FoS Formula does not account for forces related to the toe and head of an area or mass of soil with the potential for mass movement, which is; in reality the Infinite Slope formula will likely exaggerate stability conditions negatively.

The following table lists parameter values, including inferred conservative parameter values used in numerical assessments.

Table 4: Formula Parameters, Symbols & Inferred Conservative Values

Symbol	Description	Value	Unit
c_u	Effective cohesion	3.5	kPa
γ	Bulk Unit Weight of Peat	11	kN/m ³
z	Depth to failure plain	Depth of Peat	m
α	Slope Angle	Surface Topography	Degrees

2.2.8 Risk Matrices & Ranking

In assessing the risk in relation to peat stability on site it is important to rate the risk in terms of the hazard, the likelihood and the consequences if any such issue should arise. Therefore, the slope stability risk assessment considers the following parameters, which are assessed by means of a series of risk matrices (Scottish Gov., 2017).

Table 5: Parameters Included in Risk Matrices and Assessed

Category	Description
Landslide History	Considers the likelihood of landslide events occurring based on the history of the site, including the current site use.
Factor of Safety	As described above, includes the following; <ul style="list-style-type: none"> • Peat depth • Peat quality / condition • Moisture content • Incline (surface topography) • Shear strength • Bulk unit weight of peat
Substrate Topology	Identifying and qualifying variance in substrate topology and qualifying variance from theory underlining the stability formula used i.e., Infinite Slope (Parallel and no foot and head forces)
Significance of Receptor	Qualifying potential receptors in terms of significance.
Distance to Receptor	Qualifying localised proposed development areas in terms of distance to nearest receptor.

Considering the above parameters, the stability assessment follows the following steps;

1. FoS_{RAW} - Assess the site in terms of soil stability using the FoS Formula and calculate a Factor of Safety (FoS) using the *raw* data. This step is considered as preparation of the data obtained for the site i.e., translating the data to a value related to stability, and is not considered the final output of the stability assessment.
2. $FoS_{ADJUSTED}$ - Assess the FoS_{RAW} values in terms of suitability of the application of FoS Formula by considering the history of landslides in relation to the proposed site, and the topology of the substrate compared to the surface topology of the site. This is done by means of a risk matrix which qualifies the point, and also applies a coefficient for the next risk assessment step.
3. Risk Ranking RR_{SF} - The $FoS_{ADJUSTED}$ data is assessed in terms of significance of associated receptor. This is done by means of a risk matrix which qualifies the point, and also applies a coefficient for the next risk assessment step.
4. Risk Ranking RR_D - The RR_{SF} data is assessed in terms of distance to associated receptor. This is done by means of a risk matrix which qualifies the point.

Results and conclusions made by means of the above risk assessment are viewed as two tiered, that is;

1. The likelihood of a stability issue or landslide while considering the significance of the receptor (RR_{SF}).
2. The consequence of a stability issue or landslide while considering the distance to the receptor (RR_D).

For example, (1) The risk of a stability issues or landslide occurring at location X and impacting on receptor Y is negligible. (2) Considering the short distance from location X to receptor Y, in the unlikely event that an issue did arise the risk of adverse impacts effecting receptor Y is moderate.

Risk Matrices are presented in **Appendix I**.

2.2.9 Interpretation of Results.

Results of the numerical stability risk assessment are modelled / mapped and interrogated in the context of site topography, site conditions, the Project and receptor sensitivity and susceptibility. Interpretation of results in the context of the development, activity and any potential consequences is an important step of the slope stability risk assessment. It is important to consider groups of data sets and site-specific dynamics at a particular location (for example, at a proposed turbine location) and to qualitatively risk assess stability in the context of all observed site characteristics, including topography, substrate topology, geology, hydrogeology, and hydrology, etc. For example; data might indicate a single point of unacceptable FoS / stability, however this needs to be considered in context of neighbouring data and actual site conditions, such as the presence of deep peat within a localised basin confined by shallow bedrock at the surface at neighbouring points, that is; deep, “unstable” peat (by numerical model) observed to be confined by shallow bedrock does not equate to an elevated risk of a catastrophic landslide event occurring, but does equate to potential localised stability issues arising if excavating at that particular location with deep peat.

In turn, any potential stability hazard must be considered in risk assessments in terms of potential consequences to receptors, and not simply likelihood of a stability issues arising. For example, in an area with low risk in terms of stability or Factor of Safety (FoS), but immediately and directly upgradient of a sensitive receptor such as a surface water body, in the unlikely event (low risk = acceptable FoS) that a significant stability issue should arise, due to the proximity to the receiving receptor the consequences of such an event have the potential to be significant.

The following table presents the interpretation of stability risk assessment data in the context of stability, or factor of safety (FoS) (Adjusted, Scenario B) at each significant development infrastructure unit.

2.3 Subsoil & Slope Stability Risk Assessment Methodology

2.3.1 Subsoil stability numerical assessment

This stability assessment has been undertaken in a similar manner to the peat stability assessment. However, due to the limited data available (compared to number of peat depth probing locations) qualifying stability in subsoils at the Site will infer data obtained at nearest neighbour trial pit locations.

Subsoils observed on site generally are classified as follows;

- Clayey, silty, sandy, GRAVEL (or TILL) with coobles and boulders.

The undrained shear strength observed in till subsoils at the Site ranged from 15 to 180kPa (**Appendix B**). This data is not considered highly reliable due to numerous site-specific factors including particle size distribution of subsoils, particularly with high gravel / cobble content in this instance.

The undrained shear strength for inorganic silty sandy soils is typically in the range of 50 to 75kPa but is highly variable depending on the particular particle sizes and their character comprising the soil. It should be noted saturation / pore water pressure can also dramatically impact and reduce shear strength, or cohesion values in soils.

For the purpose of assessing subsoil stability for the Site a conservative undrained shear strength (effective cohesion) value will be used in numerical assessments, i.e., 40 kPa.

In situ bulk density (kg/m^3), or bulk unit weight (kN/m^3) of soils/subsoils (γ), namely silty sandy subsoils, is typically within the range of 2500 to 2700 kg/m^3 , or 24.5 to 26.5 kN/m^3 . For the purpose of assessing subsoil stability for the Site a conservative bulk unit weight value will be used in numerical assessments i.e., 27.0 kN/m^3 .

The depth to failure plane (z) is presumed to be thickness or depth of subsoils at any given sampling point being assessed. However, subsoil depths will be inferred in areas of the site with limited data. It should be noted that the failure plane can potentially be within subsoils (subsoil on subsoil movement), or the substrate i.e., weathered bedrock. In relation to the Site specifically, it is important to note the presence of iron pan. Iron pan is a layer of oxidised iron within the subsoil. The iron pan layer is relatively impermeable which can impede or significantly alter groundwater movement in the subsoils. Under the right circumstances the iron pan layer can therefore become a slip or failure plane. In such instances the failure plane has the potential to parallel to the overlying topography.

Slope angle (α) is presumed to be topographical incline measured on site / evaluated using high resolution elevation data at any given sampling point being assessed, however it should be noted that the slope angle (α) relates to the failure plane angle, which is presumed to be the peat and substrate interface, and which is presumed to be parallel to the surface when using FoS Formula (Infinite Slope Formula). In reality the underlying substrate (bedrock) is unlikely to be parallel to the surface topology. However, considering the presence of iron pan in subsoils at the site it is important to consider the potential for parallel failure planes when assessing stability at the site.

It should be noted that FoS Formula does not account for forces related to the toe and head of an area or mass of soil with the potential for mass movement, which is in reality the Infinite Slope formula will likely exaggerate stability conditions negatively.

The following table lists parameter values, including inferred conservative parameter values used in numerical assessments.

Table 6: Formula Parameters, Symbols & Inferred Conservative Values

Symbol	Description	Value	Unit
c_u	Effective cohesion	40	kPa
γ	Bulk Unit Weight of Peat	27.0	kN/m^3
z	Depth to failure plain	Depth of subsoil to bedrock	m
α	Slope Angle	Surface Topography	Degrees

2.3.2 Risk Matrices & Ranking

In assessing the risk in relation to subsoil stability on site it is important to rate the risk in terms of the hazard, the likelihood and the consequences if any such issue should arise. Therefore, the slope stability risk assessment considers the following parameters, which are assessed by means of a series of risk matrices (Scottish Gov., 2017)

Table 7: Parameters Included in Risk Matrices and Assessed

Category	Description
Landslide History	Considers the likelihood of landslide events occurring based on the history of the site, including the current site use.
Factor of Safety	As described above, includes the following; <ul style="list-style-type: none"> Subsoil depth (to failure plain)

Category	Description
	<ul style="list-style-type: none"> • Subsoil composition (PSD) • Moisture content • Incline (surface topography) • Shear strength • Bulk unit weight of subsoil
Substrate Topology	Identifying and qualifying variance in substrate topology and qualifying variance from theory underlining the stability formula used i.e., Infinite Slope (Parallel and no foot and head forces) For the purposes of considering worst case conditions (the potential for iron pan and parallel failure plains), substrate topology is considered parallel.
Significance of Receptor	Qualifying potential receptors in terms of significance.
Distance to Receptor	Qualifying localised proposed development areas in terms of distance to nearest receptor.

Considering the above parameters, the stability assessment follows the following steps;

5. FoS_{RAW} - Assess the site in terms of soil stability using the FoS Formula and calculate a Factor of Safety (FoS) using the *raw* data. This step is considered as preparation of the data obtained for the site i.e., translating the data to a value related to stability, and is not considered the final output of the stability assessment.
6. $FoS_{ADJUSTED}$ - Assess the FoS_{RAW} values in terms of suitability of the application of FoS Formula by considering the history of landslides in relation to the proposed site, and the topology of the substrate compared to the surface topology of the site. This is done by means of a risk matrix which qualifies the point, and also applies a coefficient for the next risk assessment step.
7. Risk Ranking RR_{SF} - The $FoS_{ADJUSTED}$ data is assessed in terms of significance of associated receptor. This is done by means of a risk matrix which qualifies the point, and also applies a coefficient for the next risk assessment step.
8. Risk Ranking RR_D - The RR_{SF} data is assessed in terms of distance to associated receptor. This is done by means of a risk matrix which qualifies the point.

Results and conclusions made by means of the above risk assessment are viewed as two tiered, that is;

1. The likelihood of a stability issue or landslide while considering the significance of the receptor (RR_{SF}).
2. The consequence of a stability issue or landslide while considering the distance to the receptor (RR_D).

For example, (1) The risk of a stability issues or landslide occurring at location X and impacting on receptor Y is negligible. (2) Considering the short distance from location X to receptor Y, in the unlikely event that an issue did arise the risk of adverse impacts effecting receptor Y is moderate.

Risk Matrices are presented in **Appendix I**.

3. Baseline Conditions

3.1 Site Description & History

There are no recorded landslide events in close proximity to the Site (GSI, Accessed 2021).

There were no indications of stability issues or mass movement observed on the Site during site surveys.

The Site is mapped as having areas ranging from Low Risk to High Risk in terms of Landslide Stability, that is; full spectrum of slope stability risk categories (GSI, ND). Larger areas of High-Risk landslide susceptibility are associated with relatively expansive steep slopes.

Refer to EIAR baseline section for further information (**Chapter 8: Soils and Geology**).

3.2 Site Geology

Consultation with Geological Survey Ireland Spatial Resources (GSI) indicates that the bedrock at 1:1,000,000 scale the Site is underlain by;

- Gun Point Formation (GP) – Green-grey to purple medium to fine-grained sandstones, interbedded with green and red to purple siltstones to fine sandstones.

The region contains a multitude of complex geological features however, there are no mapped faults or other significant features underlying the area of the Site.

Rocky outcrops are common within the Site Boundary.

Refer to EIAR baseline section for further information (**Chapter 8: Soils and Geology**).

3.3 Site Soils & Subsoils

Consultation with available maps (GSI) indicate that the soil type across the entire area of the Site, and the general area in the region is mostly Blanket Peat and Till derived from Devonian sandstones with several significant areas mapped as being Bedrock at Surface.

Peat depths observed on the Site are generally 'Rock' to 'shallow' with isolated pockets of moderately deep peat, however depths at most sampling points are within the range of 0.0-0.5 m and areas with deeper, particularly extremely deep peat have been avoided in terms of the Project footprint. Peat depths are mapped and presented in **Appendix A**.

Peat quality assessment (by gouge coring / trial pitting / observations at cut locations) indicate relatively moderate to high Von Post values (generally H5 to H8) across the Site.

Refer to EIAR baseline section for further information (**Chapter 8: Soils and Geology**).

3.4 Topography & Substrate Topology

The topography at and in the immediate area surrounding the Site is highly variable with multiple peaks, ridges with variable elevations and inclines. At lower elevations the topography is relatively flat or comprising of low magnitude inclines, however at mid and high elevation relative to the Site, steep high magnitude inclines are commonplace.

Site observations indicate that the substrate topology varies significantly to surface topology. Highest rates of variance are associated with areas which include deeper peat, that is; areas of deeper peat are contained with “pockets” delineated by areas or ridges of shallow bedrock. Areas with generally shallower peat have less variance from the substrate however such areas are indicatively low risk in terms of stability given the peat is shallow.

3.5 Hydrology & Climate

Three (3no.) mapped rivers run through and directly adjacent to the Site. Several extensive constructed drainage channels associated with forestry, agriculture and peat cutting activities exist at the site.

Refer to EIAR baseline section for further information (**Chapter 9: Hydrology and Hydrogeology**).

3.6 Receptors

Receptors associated with the Project footprint are generally limited to non-critical infrastructure and water bodies.

Receptors associated with the Project, which is; streams, rivers, lakes and groundwater, are considered highly sensitive receptors considering;

- ‘Good’ WFD River status and objective to protect same.
- ‘Moderate’ WFD Lake (Carrigdrohid) status and objective to restore same to at least good status by 2027.
- The numerous downgradient designations (sensitive protected areas) associated with each of the two associated catchments and the sensitive habitats and species associated with same.
- Designation of some downgradient surface water bodies and all groundwater bodies as sources of drinking water (Sullane_050).

Ultimately, all surface water and groundwater associated with the Site is considered sensitive and must be protected.

Risk to receptors must consider both the hazard, and likelihood of adversely impacting on any given sensitive receptor, and therefore parameters such as; distance from potential source of hazard to receptor, pathway directness and/or connectivity, and assimilative capacity of the receiving water body should also be considered.

Distance of proposed turbine and hard stand areas have been assessed in terms of distance to associates receptors (surface water features), the results for which are presented in **Appendix I**.

Refer to EIAR baseline section for further information (**Chapter 9: Hydrology and Hydrogeology**).

4. Site Investigation Data & Results

4.1 Peat Depth Data

Approximately 150 no. peat depth probe locations were assessed at the Site. Georeferenced and categorized peat depth locations are presented in **Appendix A**. Peat depth data is presented in **Appendix B**. Number of probe locations by Depth Category are presented in **Table 8**.

Table 8: Peat Depth Probe Points per Depth Category

Peat Depth Category	No.
A – Rock (0.00-0.01 m)	16
B – Very Shallow (0.01-0.5 m)	92
C – Shallow (0.5-2.0 m)	66
D – Moderately Deep (2.0-3.5m)	12
E – Deep (3.5-5.0 m)	1
F – Very Deep (>5.0 m)	0
TOTAL	187 (21 Inferred)

4.2 Trial Pit Data

A total of 16 no. Trial Pits were completed, logged and sampled at the Site. Trial Pit and Borehole locations are presented in **Appendix C**. Trial Pit Logs are presented in **Appendix D**. Trial Pit and Site Investigation Photos are presented in **Appendix E**. A total of 3 no. subsoil samples were obtained from the Site and tested for particle size distribution (PSD). Subsoil laboratory certificates are presented in **Appendix G**.

Particle Size Distribution (PSD) Soil Description results for subsoils (BS 1377: Part 2: 1990: Clause 9) at the site are presented in **Table 9**. Note: cobble size particles observed on trial pit log sheets and have likely been screened out to a degree at the time of sampling.

Table 9: Reported Subsoil Description (PSD)

Sample ID	Cobbles (%)	Gravel (%)	Sand (%)	Silt & Clay (%)	Description
TP03-A2 (SS1)	0.0	43.0	32.0	25.0	Very clayey very sandy GRAVEL
TP08-A2 (SS1)	0.0	50.0	19.0	31.0	Slightly sandy gravelly CLAY
TP11-A2 (SS1)	0.0	51.0	26.0	22.0	Very clayey very sandy GRAVEL

Cobbles were observed on site and were likely screened out at the time of sampling. Further details are presented in **Appendix D**. Iron pan was observed in several trial pits as listed in **Appendix H**, and presented in **Appendix C**, **Appendix D** and **Appendix E**.

4.3 Borehole Data

A total of 1 no. rotary core borehole was completed, logged, and sampled at the Site. Borehole logs are presented in **Appendix F**. Drill logs indicate that;

- Bedrock underlying the site is described as SILTSTONE (BH011)
- Bedrock shows minor signs of weathering.
- Driller notes water strike at BH011 at ~2.50m bGL likely perched groundwater on top of unweathered bedrock. .

Siltstone is mainly comprised of silt-sized particles. Silt-sized particles range between 0.002 and 0.063 millimeters in diameter (BS 5930). They are intermediate in size between coarse clay on the small side and fine sand on the large side.

Bedrock cores obtained were tested for Unconfined Compressive Strength (UCS) and Point Load Strength (PL). Rock core testing laboratory certificates are presented in **Appendix F**. Unconfined Compressive Strength (UCS) results presented in **Table 10** indicate bedrock underlying the site is considered weak.

Table 10: Bedrock Core Laboratory Strength Testing Results

Parameter	(Unit)	BH011
UCS Results	<i>Kn</i>	23.3
UCS Results	<i>MPa</i>	5.17
Rock Strength (UCS MPa)	<i>BS 5930</i> <i>BS EN ISO 14689</i>	Weak

4.4 Peat Stability Risk Assessment Results

Review of peat stability assessment result data and maps as presented in **Appendix I** indicate that the factor of safety is generally acceptable and very low to low stability risk across the site with the exception of minor isolated areas or pockets of deeper peat.

Summary of risk at the site under varying conditions and scenarios is presented in in the following tables.

Table 11: Factor of Safety (Adjusted) at Peat Probe Locations

	Acceptable	Marginally Stable	Unstable
FoS (Adj.) Scenario A	149	1	0
FoS (Adj.) Scenario B	118	24	8

Table 12: Risk Ranking (Distance) at Peat Probe Locations

	Very Low	Low	Moderate	High
RR (Dist.) Scenario A	104	11	34	1
RR (Dist.) Scenario B	81	27	37	5

Areas of elevated stability risk, even at a localised scale, are considered geo-hazards requiring mitigation. Geo-hazards are presented in **Appendix H**.



The following plates present the available peat data per proposed turbine locations, including the results of numerical model stability risk assessment.

SI Appendix B - Peat & Subsoil Survey Database
Inchmore WF, Co. Cork

Prepared by: SK 07/02/2023
RSK File Ref.: 003679-00.xls

Sample / Test Category	Sample / Test ID No.	Association	ITM Easting	ITM Northing	Thickness / Depth of peat	Classification of Thickness / Depth of peat	Slope (Extracted from GDEM)	Note	Scenario A		Scenario B		Scenario A		Scenario B		Scenario A		Scenario B		RR ₀ Risk Ranking Accounting for Distance to Sensitive Receptors	RR ₀ Risk Ranking Accounting for Distance to Sensitive Receptors
									FOS _{RAW} Factor of Safety (FoS) for Peat Stability	FOS _{ADJ} Adjusted Factor of Safety (FoS) for Peat Stability	FOS _{RAW} Factor of Safety (FoS) for Peat Stability	FOS _{ADJ} Adjusted Factor of Safety (FoS) for Peat Stability	RR _{RF} Ranking Risk re Potential for Adverse Consequences on Sensitive Receptors	RR _{RF} Ranking Risk re Potential for Adverse Consequences on Sensitive Receptors	RR _{RF} Ranking Risk re Potential for Adverse Consequences on Sensitive Receptors	RR _{RF} Ranking Risk re Potential for Adverse Consequences on Sensitive Receptors	Risk Category	Risk Category				
Depth Probe	DP001	TI	50242.9	57902.9	0.1	Very Shallow (0.01-0.5m)	6.76569		3163	2.09	2.0	0.0	3163	2.09	2.0	10	420.2	10	420.2	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP003	TI	50246.9	57904.9	0.1	Very Shallow (0.01-0.5m)	6.76569		3163	2.09	2.0	0.0	3163	2.09	2.0	10	420.2	10	420.2	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP004	TI	50280.9	57902.9	0.2	Very Shallow (0.01-0.5m)	2.54332		58.83	5.88	2.0	0.0	58.83	5.88	2.0	10	414.1	10	414.1	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP005	TI	50288.9	57904.9	0.1	Very Shallow (0.01-0.5m)	5.8395		31.14	2.83	2.0	0.0	31.14	2.83	2.0	10	375.9	10	375.9	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP006	TI	50289.9	57905.9	0.1	Very Shallow (0.01-0.5m)	0.00302	Floody outcrop east west strap	16.44	1.66	2.0	0.0	16.44	1.66	2.0	10	384.1	10	384.1	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP008	TI	50401.9	57904.9	0.7	Shallow (0.5-2.0m)	6.049	Cut peat Photo 1160	4.37	1.70	2.0	0.0	4.37	1.70	2.0	10	340.9	10	340.9	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP007	TI	50218.9	57897.9	0.8	Shallow (0.5-2.0m)	2.0186		1165	4.37	2.0	0.0	1165	4.37	2.0	10	363.6	10	363.6	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP009	TI	50240.9	57905.9	2.1	Moderately Deep (2.0-3.5m)	4.97205		1.75	1.18	2.0	0.0	1.75	1.18	2.0	10	315.1	10	315.1	A - Very Low Risk	B - Low Risk	
Depth Probe	DP008	TI	50247.9	57887.9	1.0	Shallow (0.5-2.0m)	3.0206		6.88	2.93	2.0	0.0	6.88	2.93	2.0	10	343.2	10	343.2	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP010	TI	50450.9	57904.9	0.0000	A-Flocc (0.0m)	0.07050	Flocc	29749.24	3974	2.0	0.0	29749.24	3974	2.0	10	397.4	10	397.4	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP011	TI	5048.9	57902.9	1.2	Shallow (0.5-2.0m)	0.87893	Floody adjacent	17.23	8.43	2.0	0.0	17.23	8.43	2.0	10	348.9	10	348.9	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP014	TI	5120.0	57907.6	1.1	Shallow (0.5-2.0m)	2.5434		6.80	3.45	2.0	0.0	6.80	3.45	2.0	10	345.0	10	345.0	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP015	TI	50219.9	57921.9	0.0000	A-Flocc (0.0m)	6.05488	Flocc	39324.02	3.80	2.0	0.0	39324.02	3.80	2.0	10	345.0	10	345.0	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP016	TI	50254.9	57940.0	0.0000	A-Flocc (0.0m)	2.08887	Flocc	87391.72	8.73	2.0	0.0	87391.72	8.73	2.0	10	387.7	10	387.7	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP017	TI	50252.0	57900.0	0.0000	A-Flocc (0.0m)	4.21701	Flocc	4362.2	4.49	2.0	0.0	4362.2	4.49	2.0	10	337.1	10	337.1	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP018	TI	50259.9	57902.0	0.0000	A-Flocc (0.0m)	3.27531	Flocc	55781.75	5.56	2.0	0.0	55781.75	5.56	2.0	10	388.7	10	388.7	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP019	TI	50252.0	57920.9	2.1	Moderately Deep (2.0-3.5m)	4.96234		1.70	1.21	2.0	0.0	1.70	1.21	2.0	10	490.2	10	490.2	A - Very Low Risk	B - Low Risk	
Depth Probe	DP021	TI	50243.0	57900.0	0.0000	A-Flocc (0.0m)	3.9734	Flocc	5424.96	5.44	2.0	0.0	5424.96	5.44	2.0	10	249.1	10	249.1	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP022	TI	50244.9	57909.0	2.0	Moderately Deep (2.0-3.5m)	2.00639		3.50	2.63	2.0	0.0	3.50	2.63	2.0	10	350.0	10	350.0	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP024	TI	50246.9	57907.0	2.9	Moderately Deep (2.0-3.5m)	2.39497		2.72	2.01	2.0	0.0	2.72	2.01	2.0	10	337.4	10	337.4	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP025	TI	50248.9	57906.9	1.5	Shallow (0.5-2.0m)	2.00889	Floody adjacent photo	6.06	3.84	2.0	0.0	6.06	3.84	2.0	10	337.4	10	337.4	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP026	TI	50250.9	57987.0	2.0	Moderately Deep (2.0-3.5m)	1.48887		4.90	3.90	2.0	0.0	4.90	3.90	2.0	10	386.2	10	386.2	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP027	TI	50250.9	57908.0	2.2	Moderately Deep (2.0-3.5m)	3.96734		2.47	1.70	2.0	0.0	2.47	1.70	2.0	10	278.6	10	278.6	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP028	TI	50254.9	57902.0	2.4	Moderately Deep (2.0-3.5m)	2.39497		2.54	1.91	2.0	0.0	2.54	1.91	2.0	10	382.9	10	382.9	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP029	TI	50256.9	57907.9	2.5	Moderately Deep (2.0-3.5m)	2.00888	Floody adjacent photo	3.84	2.63	2.0	0.0	3.84	2.63	2.0	10	318.1	10	318.1	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP030	TI	50256.9	57909.0	3.0	Moderately Deep (2.0-3.5m)	4.90788		1.48	1.11	2.0	0.0	1.48	1.11	2.0	10	412.9	10	412.9	A - Very Low Risk	B - Low Risk	
Depth Probe	DP031	TI	50270.9	57917.0	2.1	Moderately Deep (2.0-3.5m)	1.8938		4.62	3.13	2.0	0.0	4.62	3.13	2.0	10	384.1	10	384.1	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP032	TI	50280.9	57909.0	0.0000	A-Flocc (0.0m)	2.321		2.52	2.32	2.0	0.0	2.52	2.32	2.0	10	287.2	10	287.2	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP033	TI	50291.9	57902.0	1.1	Shallow (0.5-2.0m)	7.2024		-9.25	-0.93	2.0	0.0	-9.25	-0.93	2.0	10	342.9	10	342.9	A - Very Low Risk	B - Low Risk	
Depth Probe	DP034	TI	50291.9	57907.0	0.0000	A-Flocc (0.0m)	6.88087	Floody adjacent	26760.80	2.67	2.0	0.0	26760.80	2.67	2.0	10	380.0	10	380.0	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP035	TI	50294.9	57901.0	2.0	Moderately Deep (2.0-3.5m)	4.82378		4.48	3.01	2.0	0.0	4.48	3.01	2.0	10	346.1	10	346.1	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP036	TI	50245.9	57902.0	0.0000	A-Flocc (0.0m)	3.96095		1.75	1.28	2.0	0.0	1.75	1.28	2.0	10	388.8	10	388.8	A - Very Low Risk	B - Low Risk	
Depth Probe	DP037	TI	50245.9	57909.0	2.0	Moderately Deep (2.0-3.5m)	4.97525	Flocc	37974.96	3.75	2.0	0.0	37974.96	3.75	2.0	10	344.1	10	344.1	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP038	TI	50245.9	57903.0	0.0000	A-Flocc (0.0m)	4.97525	Flocc	37974.96	3.75	2.0	0.0	37974.96	3.75	2.0	10	230.0	10	230.0	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP039	TI	50245.9	57901.0	0.2	Very Shallow (0.01-0.5m)	6.43		2.57	2.57	2.0	0.0	2.57	2.57	2.0	10	270.1	10	270.1	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP040	TI	50245.9	57901.0	1.4	Shallow (0.5-2.0m)	4.5986		2.80	1.63	2.0	0.0	2.80	1.63	2.0	10	318.5	10	318.5	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP041	TI	50247.9	57901.0	1.1	Shallow (0.5-2.0m)	4.5986		1.82	1.21	2.0	0.0	1.82	1.21	2.0	10	365.1	10	365.1	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP043	TI	50267.9	57907.9	1.2	Shallow (0.5-2.0m)	4.62383		3.23	1.86	2.0	0.0	3.23	1.86	2.0	10	287.7	10	287.7	A - Very Low Risk	A - Very Low Risk	
Depth Probe	DP044	TI	50271.9	57988.0	0.0000	A-Flocc (0.0m)	8.5440	Flocc	21866.48	2.17	2.0	0.0	21866.48	2.17	2.0	10	364.1	10	364.1	A - Very Low Risk	A - Very Low Risk	
Trail Pit	YP1	TI	50265.9	57919.0	1.0	Shallow (0.5-2.0m)	6.30381		23.9	2.65	2.0	0.0	23.9	2.65	2.0	10	329.1	10	329.1	A - Very Low Risk	A - Very Low Risk	
Trail Pit	YP4	TI	50254.9	57904.5	3.0	Moderately Deep (2.0-3.5m)	2.39497		1.48	1.18	2.0	0.0	1.48	1.18	2.0	10	344.0	10	344.0	A - Very Low Risk	A - Very Low Risk	
Trail Pit	YP5	TI	50249.9	57909.0	1.0	Shallow (0.5-2.0m)	3.0206		16.50	4.50	2.0	0.0	16.50	4.50	2.0	10	291.0	10	291.0	A - Very Low Risk	A - Very Low Risk	
Trail Pit	YP8	TI	50223.9	57888.0	0.2	Very Shallow (0.01-0.5m)	4.67018	Groundwater logged at base	8.80	3.27	2.0	0.0	8.80	3.27	2.0	10	482.8	10	482.8	A - Very Low Risk	A - Very Low Risk	

Plate 1: Peat Data & Risk Assessment Results – T1



SI Appendix B - Peat & Subsoil Survey Database
Inchamore WF, Co. Cork

Prepared by: SK 07/02/2023
RSK File Ref.: 603679-00.xls

Sample / Test Category	Sample / Test Point ID No.	Association	ITM Easting	ITM Northing	Thickness / Depth of peat	Classification of Thickness / Depth of peat	Slope (Extracted from GDEM)	Note	Scenario A		Scenario B		Scenario A		Scenario B		Scenario A		Scenario B		Scenario A		Scenario B	
									FOS _{RAW}	FOS _{RAW}	FOS _{ADJ}	FOS _{ADJ}	RR _{SF}	RR _{SF}	RR _D	RR _D	RR _{SF}	RR _{SF}	RR _D	RR _D				
									Factor of Safety (FoS) for Peat Stability	Factor of Safety (FoS) for Peat Stability	Adjusted Factor of Safety (FoS) for Peat Stability	Adjusted Factor of Safety (FoS) for Peat Stability	Ranking Potential for Adverse Consequences on Sensitive Receptors	Ranking Potential for Adverse Consequences on Sensitive Receptors	Risk Accounting for Distance to Sensitive Receptors	Risk Accounting for Distance to Sensitive Receptors	Ranking Potential for Adverse Consequences on Sensitive Receptors	Ranking Potential for Adverse Consequences on Sensitive Receptors	Risk Accounting for Distance to Sensitive Receptors	Risk Accounting for Distance to Sensitive Receptors				
Depth Probe	DP001	T2	512658.9	578676.9	0.3	B1-Veg Shallow (0.0-0.5m)	82.254	Rookly adjacent	6.43	1.48	2.0	0.6	6.43	1.48	2.0	1.0	1275	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP002	T2	512707.1	578640.5	0.0001	A-Rock (0.0m)	9.5804		2667.51	2.35	2.0	0.0	2667.51	2.35	2.0	1.0	153	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP006	T2	512722.9	578686	0.0001	A-Rock (0.0m)	9.6086		2634.52	2.05	2.0	0.0	2634.52	2.05	2.0	1.0	351	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP057	T2	512776.8	578593	0.2	B1-Veg Shallow (0.0-0.5m)	9.6824		8.67	1.61	2.0	0.0	8.67	1.61	2.0	1.0	153	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP059	T2	512806.8	578622.9	0.1	B1-Veg Shallow (0.0-0.5m)	10.2084	Rookly adjacent	164	1.64	2.0	0.0	164	1.64	2.0	1.0	62	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP061	T2	512818	578686	0.1	B1-Veg Shallow (0.0-0.5m)	10.1928	Rookly adjacent	16.35	1.66	2.0	0.0	16.35	1.66	2.0	1.0	74	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP062	T2	512828	5786918	0.2	B1-Veg Shallow (0.0-0.5m)	10.6979		1.78	1.78	2.0	0.0	1.78	1.78	2.0	1.0	171	2.0	2.0	A-Very Low Risk	2.0	A-Very Low Risk		
Depth Probe	DP063	T2	512838.8	578687.8	0.1	B1-Veg Shallow (0.0-0.5m)	8.8976	Rookly adjacent	20.53	1.81	2.0	0.0	20.53	1.81	2.0	1.0	24	4.0	4.0	C-Moderate Risk	4.0	C-Moderate Risk		
Depth Probe	DP065	T2	512846.8	578695.8	0.1	B1-Veg Shallow (0.0-0.5m)	7.5029	Rookly adjacent	24.53	2.23	2.0	0.0	24.53	2.23	2.0	1.0	20	4.0	4.0	C-Moderate Risk	4.0	C-Moderate Risk		
Depth Probe	DP068	T2	512862	578676	0.1	B1-Veg Shallow (0.0-0.5m)	7.0707		26.05	2.31	2.0	0.0	26.05	2.31	2.0	1.0	324	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP067	T2	512864.4	578630.5	0.0001	A-Rock (0.0m)	8.8902	Rookly adjacent	2682.11	2.03	2.0	0.0	2682.11	2.03	2.0	1.0	820	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP068	T2	512902.1	578613	0.3	B1-Veg Shallow (0.0-0.5m)	8.8902		4.44	1.69	2.0	0.0	4.44	1.69	2.0	1.0	84	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP070	T2	512940	578538	0.5	B1-Veg Shallow (0.0-0.5m)	11.5304		3.30	1.50	2.0	0.0	3.30	1.50	2.0	1.0	600	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP072	T2	512948	578644.2	0.5	B1-Veg Shallow (0.0-0.5m)	7.888	INFERRED	4.79	1.60	2.0	0.0	4.79	1.60	2.0	1.0	102	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP073	T2	512957.2	578664	0.5	B1-Veg Shallow (0.0-0.5m)	9.0682	INFERRED	2.45	2.35	2.0	0.0	2.45	2.35	2.0	1.0	600	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP074	T2	512985.7	578687	0.5	B1-Veg Shallow (0.0-0.5m)	6.5471	INFERRED	5.62	1.87	2.0	0.0	5.62	1.87	2.0	1.0	57	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP075	T2	512986.3	578665.5	0.5	B1-Veg Shallow (0.0-0.5m)	12.3688	INFERRED	3.04	1.81	2.0	0.0	3.04	1.81	2.0	1.0	61	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP076	T2	513016	578622.7	0.5	B1-Veg Shallow (0.0-0.5m)	13.0578	INFERRED	7.23	1.20	2.0	0.0	7.23	1.20	2.0	1.0	40	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP077	T2	51308.4	578638.4	0.5	B1-Veg Shallow (0.0-0.5m)	13.3682	INFERRED	2.70	0.51	4.0	0.0	2.70	0.51	4.0	1.0	40	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP078	T2	512922.9	578637.6	0.5	B1-Veg Shallow (0.0-0.5m)	37.503	INFERRED	3.52	1.21	2.0	0.0	3.52	1.21	2.0	1.0	40	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP079	T2	512928	578651.5	0.5	B1-Veg Shallow (0.0-0.5m)	37.503	INFERRED	3.82	1.27	2.0	0.0	3.82	1.27	2.0	1.0	40	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP080	T2	512944.9	578651	0.5	B1-Veg Shallow (0.0-0.5m)	6.0271	INFERRED	6.11	2.04	2.0	0.0	6.11	2.04	2.0	1.0	64	4.0	4.0	C-Moderate Risk	4.0	C-Moderate Risk		
Depth Probe	DP081	T2	512953.3	578672.8	0.5	B1-Veg Shallow (0.0-0.5m)	6.0271	INFERRED	6.11	2.04	2.0	0.0	6.11	2.04	2.0	1.0	64	4.0	4.0	C-Moderate Risk	4.0	C-Moderate Risk		
Depth Probe	DP082	T2	512959.9	578658.4	0.5	B1-Veg Shallow (0.0-0.5m)	7.2663	INFERRED	4.83	1.63	2.0	0.0	4.83	1.63	2.0	1.0	28	4.0	4.0	C-Moderate Risk	4.0	C-Moderate Risk		
Depth Probe	DP083	T2	512962.8	578704.8	0.5	B1-Veg Shallow (0.0-0.5m)	7.5247	INFERRED	4.30	1.63	2.0	0.0	4.30	1.63	2.0	1.0	46	4.0	4.0	C-Moderate Risk	4.0	C-Moderate Risk		
Depth Probe	DP084	T2	512988.8	578785	0.5	B1-Veg Shallow (0.0-0.5m)	12.3276	INFERRED	3.85	1.62	2.0	0.0	3.85	1.62	2.0	1.0	40	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP085	T2	512905.9	578702	0.5	B1-Veg Shallow (0.0-0.5m)	14.0293	INFERRED	2.91	0.82	4.0	0.0	2.91	0.82	4.0	1.0	40	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP086	T2	512918	578632	0.5	B1-Veg Shallow (0.0-0.5m)	14.7208	INFERRED	2.60	0.88	4.0	0.0	2.60	0.88	4.0	1.0	40	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Depth Probe	DP087	T2	512936.3	578679	0.4	B1-Veg Shallow (0.0-0.5m)	7.8622	INFERRED	4.64	1.53	2.0	0.0	4.64	1.53	2.0	1.0	61	4.0	4.0	C-Moderate Risk	4.0	C-Moderate Risk		
Depth Probe	DP088	T2	512942	578670.6	0.5	B1-Veg Shallow (0.0-0.5m)	6.0271	INFERRED	6.11	2.04	2.0	0.0	6.11	2.04	2.0	1.0	64	4.0	4.0	C-Moderate Risk	4.0	C-Moderate Risk		
Depth Probe	DP089	T2	512954.9	578693.9	0.5	B1-Veg Shallow (0.0-0.5m)	6.0271	INFERRED	6.11	2.04	2.0	0.0	6.11	2.04	2.0	1.0	64	4.0	4.0	C-Moderate Risk	4.0	C-Moderate Risk		
Depth Probe	DP090	T2	512974.7	578690.2	0.5	B1-Veg Shallow (0.0-0.5m)	7.7752	INFERRED	4.75	1.53	2.0	0.0	4.75	1.53	2.0	1.0	28	4.0	4.0	C-Moderate Risk	4.0	C-Moderate Risk		
Depth Probe	DP091	T2	512916	578638.8	0.5	B1-Veg Shallow (0.0-0.5m)	7.7946	INFERRED	4.74	1.53	2.0	0.0	4.74	1.53	2.0	1.0	28	4.0	4.0	C-Moderate Risk	4.0	C-Moderate Risk		
Total Pts	TP1		512701	578602	0.4	B1-Veg Shallow (0.0-0.5m)	12.4957		2.71	1.08	2.0	0.0	2.71	1.08	2.0	1.0	60	2.0	2.0	B-Low Risk	2.0	B-Low Risk		
Total Pts	TP2		512687	578632	0.5	B1-Veg Shallow (0.0-0.5m)	9.4635		6.84	1.51	2.0	0.0	6.84	1.51	2.0	1.0	38	4.0	4.0	C-Moderate Risk	4.0	C-Moderate Risk		

Plate 2: Peat Data & Risk Assessment Results – T2



SI Appendix B - Peat & Subsoil Survey Database
Inchamore WF, Co. Cork

Prepared by: SK 01/02/2023
RSK File Ref.: 603679-00.xls

Sample / Test Category	Test Point ID No.	Association	ITM Easting	ITM Northing	Thickness / Depth of peat	Classification of Thickness / Depth of peat	Slope (Extracted from GPR)	Note	Scenario A		Scenario B		Scenario A		Scenario B		Scenario A		Scenario B		Scenario A		Scenario B		
									FoS _{RAW}	FoS _{ADJ}	FoS _{RAW}	FoS _{ADJ}	RR _{RF}	RR _{RF}	RR _D	RR _D	RR _{RF}	RR _{RF}	RR _D	RR _D					
									Factor of Safety (FoS) for Peat Stability	Adjusted Factor of Safety (FoS) for Peat Stability	Factor of Safety (FoS) for Peat Stability	Adjusted Factor of Safety (FoS) for Peat Stability	Potential for Adverse Consequences on Sensitive Receptors	Potential for Adverse Consequences on Sensitive Receptors	Risk Ranking Accounting for Distance to Sensitive Receptors	Risk Ranking Accounting for Distance to Sensitive Receptors	Potential for Adverse Consequences on Sensitive Receptors	Potential for Adverse Consequences on Sensitive Receptors	Risk Ranking Accounting for Distance to Sensitive Receptors	Risk Ranking Accounting for Distance to Sensitive Receptors					
Depth Probe	DP088	T3	51786.9	87991.9	0.3	C - Shallow (0.5-2.0m)	8.2729		2.48	1.85	2.0	0.0	2.48	1.85	2.0	0.0	4.0	2.0	2.67	1.0	A - Very Low Risk	20	B - Low Risk		
Depth Probe	DP089	T3	51787.0	87992.0	1.0	C - Shallow (0.5-2.0m)	12.6644		1.03	0.75	1.0	0.0	1.03	0.75	1.0	0.0	3.0	4.0	3.71	1.0	A - Very Low Risk	41	C - Moderate Risk		
Depth Probe	DP084	T3	51287	87905	0.4	Very Shallow (0.1-0.5m)	10.7688	Deeply eroded drain	4.38	1.58	2.0	0.0	4.38	1.58	2.0	0.0	4.0	2.0	4.83	2.0	B - Low Risk	40	C - Moderate Risk		
Depth Probe	DP085	T3	51294.6	87905.9	0.9	C - Shallow (0.5-2.0m)	9.2446	Cut peat photo	2.25	1.59	2.0	0.0	2.25	1.59	2.0	0.0	4.0	2.0	4.23	1.0	A - Very Low Risk	35	B - Low Risk		
Depth Probe	DP071	T3	51287	87838	0.0001	A - Peat (0.0m)	4.8792	Cut peat	3765.14	3.75	2.0	0.0	3765.14	3.75	2.0	0.0	1.0	1.0	442.0	1.0	A - Very Low Risk	1	A - Very Low Risk		
Depth Probe	DP072	T3	51286	87838	0.3	C - Shallow (0.5-2.0m)	4.8792	Unrot peat	6.28	2.35	2.0	0.0	6.28	2.35	2.0	0.0	1.0	1.0	442.0	1.0	A - Very Low Risk	1	A - Very Low Risk		
Depth Probe	DP073	T3	51286	87838	0.0001	A - Peat (0.0m)	4.8679	Cut peat	3092.29	3.02	2.0	0.0	3092.29	3.02	2.0	0.0	1.0	1.0	403.0	1.0	A - Very Low Risk	1	A - Very Low Risk		
Depth Probe	DP074	T3	51286	87829	0.4	C - Shallow (0.5-2.0m)	6.0629	Unrot peat	8.03	1.89	2.0	0.0	8.03	1.89	2.0	0.0	1.0	1.0	402.0	1.0	A - Very Low Risk	1	A - Very Low Risk		
Depth Probe	DP075	T3	51286	87806	0.1	Very Shallow (0.1-0.5m)	8.0638	Cut peat	23.06	2.30	2.0	0.0	23.06	2.30	2.0	0.0	1.0	1.0	352.0	1.0	A - Very Low Risk	1	A - Very Low Risk		
Depth Probe	DP076	T3	51287	87816	1.5	C - Shallow (0.5-2.0m)	8.0638	Unrot peat	1.54	0.92	4.0	0.0	1.54	0.92	4.0	0.0	4.0	4.0	393.0	1.0	A - Very Low Risk	4	C - Moderate Risk		
Depth Probe	DP077	T3	51000	87833	0.1	Very Shallow (0.1-0.5m)	7.8543		23.47	2.13	2.0	0.0	23.47	2.13	2.0	0.0	1.0	1.0	317.0	1.0	A - Very Low Risk	1	A - Very Low Risk		
Depth Probe	DP078	T3	51000	87834	1.1	C - Shallow (0.5-2.0m)	7.8543	Unrot peat, inferred	2.13	1.32	2.0	0.0	2.13	1.32	2.0	0.0	1.0	1.0	4.1	2.0	385.0	1.0	A - Very Low Risk	2	B - Low Risk
Depth Probe	DP079	T3	51001	87834	1.1	C - Shallow (0.5-2.0m)	7.8543	Cut peat on fill	1.80	0.83	4.0	0.0	1.80	0.83	4.0	0.0	1.0	1.0	3.0	4.0	218.0	1.0	A - Very Low Risk	4	C - Moderate Risk
Depth Probe	DP080	T3	51005.0	87869.0	0.4	Very Shallow (0.1-0.5m)	10.7688	Cut peat photo	4.38	1.58	2.0	0.0	4.38	1.58	2.0	0.0	1.0	1.0	4.1	2.0	392.0	1.0	A - Very Low Risk	3	B - Low Risk
Depth Probe	DP081	T3	51902	87904	0.5	C - Shallow (0.5-2.0m)	10.7987	Top of steep slope	6.59	1.48	2.0	0.0	6.59	1.48	2.0	0.0	1.0	1.0	2.94	1.0	A - Very Low Risk	4	C - Moderate Risk		
Depth Probe	DP082	T3	51002.0	87869.0	1.0	C - Shallow (0.5-2.0m)	11.5727		1.57	0.78	4.0	0.0	1.57	0.78	4.0	0.0	1.0	1.0	4.4	2.0	395.0	1.0	A - Very Low Risk	4	C - Moderate Risk
Depth Probe	DP083	T3	51364	87991	0.5	Very Shallow (0.1-0.5m)	11.5727	Up out of drain	3.14	1.05	2.0	0.0	3.14	1.05	2.0	0.0	1.0	1.0	4.1	2.0	323.0	1.0	A - Very Low Risk	2	B - Low Risk
Depth Probe	DP084	T3	51004	87992	1.3	C - Shallow (0.5-2.0m)	5.3364		2.65	1.50	2.0	0.0	2.65	1.50	2.0	0.0	1.0	1.0	3.91	1.0	A - Very Low Risk	1	A - Very Low Risk		
Depth Probe	DP085	T3	51007.0	87888.0	0.1	Very Shallow (0.1-0.5m)	10.5263		1.72	1.13	2.0	0.0	1.72	1.13	2.0	0.0	1.0	1.0	2.0	1.0	207.0	1.0	A - Very Low Risk	1	A - Very Low Risk
Depth Probe	DP086	T3	51008.0	87909.0	0.9	C - Shallow (0.5-2.0m)	7.8291		1.59	0.94	2.0	0.0	1.59	0.94	2.0	0.0	1.0	1.0	4.1	2.0	317.0	1.0	A - Very Low Risk	2	B - Low Risk
Depth Probe	DP087	T3	51008.0	87909.0	1.8	C - Shallow (0.5-2.0m)	7.8291		1.58	0.93	4.0	0.0	1.58	0.93	4.0	0.0	1.0	1.0	3.0	4.0	317.0	1.0	B - Low Risk	4	C - Moderate Risk
Depth Probe	DP088	T3	51008.0	87910.0	0.3	C - Shallow (0.5-2.0m)	8.3395		2.38	1.05	2.0	0.0	2.38	1.05	2.0	0.0	1.0	1.0	4.1	2.0	381.0	1.0	A - Very Low Risk	2	B - Low Risk
Depth Probe	DP089	T3	51034.0	87900.0	1.1	C - Shallow (0.5-2.0m)	8.4433		1.89	1.04	2.0	0.0	1.89	1.04	2.0	0.0	1.0	1.0	4.1	2.0	384.0	1.0	A - Very Low Risk	2	B - Low Risk
Depth Probe	DP090	T3	51048	87989	0.3	C - Shallow (0.5-2.0m)	7.1033		2.89	1.28	2.0	0.0	2.89	1.28	2.0	0.0	1.0	1.0	2.0	1.0	323.0	1.0	A - Very Low Risk	1	A - Very Low Risk
Depth Probe	DP091	T3	51061.0	87961.0	1.1	C - Shallow (0.5-2.0m)	8.0566		2.77	1.45	2.0	0.0	2.77	1.45	2.0	0.0	1.0	1.0	2.0	1.0	301.0	1.0	A - Very Low Risk	1	A - Very Low Risk
Trial Pit	TP07	T3	51260	87897	0.2	Very Shallow (0.1-0.5m)	11.5828		8.24	1.27	2.0	0.0	8.24	1.27	2.0	0.0	1.0	1.0	1.0	1.0	853.0	1.0	A - Very Low Risk	1	A - Very Low Risk
Trial Pit	TP08	T3	51274	87897	0.2	Very Shallow (0.1-0.5m)	8.2795		8.30	1.65	2.0	0.0	8.30	1.65	2.0	0.0	1.0	1.0	1.0	1.0	440.0	1.0	A - Very Low Risk	1	A - Very Low Risk
Trial Pit	TP09	T3	51269	87901	0.2	Very Shallow (0.1-0.5m)	7.4724		10.02	2.00	2.0	0.0	10.02	2.00	2.0	0.0	1.0	1.0	1.0	1.0	368.0	1.0	A - Very Low Risk	1	A - Very Low Risk

Plate 3: Peat Data & Risk Assessment Results – T3



SI Appendix B - Peat & Subsoil Survey Database
Inchamore WF, Co. Cork

Prepared by: SK 07/02/2023
RSK File Ref.: 603679-00.xls

Sample / Test Category	Sampl e / Test Point ID No.	Association	ITM Easting	ITM Northin g	Thickne ss / Depth of peat	Classification of peat	Slope (Extract ed from GDEM)	Note	Scenario A		Scenario B		Significant Feature Ranking Coefficient	Scenario A		Scenario B		Distance to Sensitive Receptor	Scenario A		Scenario B			
									FOS _{RAW}	FOS _{ADJ}	FOS _{RAW}	FOS _{ADJ}		RR _{SF}	RR _{SF}	RR _D	RR _D							
									Factor of Safety (FoS) for Peat Stability	Adjusted Factor of Safety (FoS) for Peat Stability	Factor of Safety (FoS) for Peat Stability	Adjusted Factor of Safety (FoS) for Peat Stability		Ranking Risk re Potential for Adverse Consequences on Sensitive Receptors	Ranking Risk re Potential for Adverse Consequences on Sensitive Receptors	Risk Ranking Accounting for Distance to Sensitive Receptors	Risk Ranking Accounting for Distance to Sensitive Receptors							
Depth Probe	DP14	T4	516859.7	579022.9	0.2	Very Shallow (0.01-0.1m)	11.0727		0.37	1.99	2.0	0.0	0.37	1.99	2.0	0.0	1919	10	A	Very Low Risk	10	A	Very Low Risk	
Depth Probe	DP18	T4	516878.7	578822.9	1	C - Shallow (0.5-2.0m)	1147724		1.82	0.82	4.0	2.0	0.0	1.82	0.82	4.0	2.0	268	10	A	Very Low Risk	4.0	C	Moderate Risk
Depth Probe	DP16	T4	516892.1	578903.9	0.1	Very Shallow (0.01-0.1m)	6.62442		2777.1	2.92	2.0	0.0	2777.1	2.92	2.0	0.0	209	10	A	Very Low Risk	10	A	Very Low Risk	
Depth Probe	DP17	T4	516882.7	578820.9	1	C - Shallow (0.5-2.0m)	1147724		1.82	0.82	4.0	2.0	0.0	1.82	0.82	4.0	2.0	268	10	A	Very Low Risk	4.0	C	Moderate Risk
Depth Probe	DP18	T4	516892.1	578902	0.1	Very Shallow (0.01-0.1m)	6.62442		2777.1	2.92	2.0	0.0	2777.1	2.92	2.0	0.0	209	10	A	Very Low Risk	10	A	Very Low Risk	
Depth Probe	DP19	T4	516893.9	578879.9	0.5	Very Shallow (0.01-0.1m)	9.78785	INFERRED, TRACK SIDE	3.79	1.26	2.0	0.0	3.79	1.26	2.0	0.0	43	2.0	B	Low Risk	2.0	B	Low Risk	
Depth Probe	DP20	T4	51674.7	579069.9	0.1	Very Shallow (0.01-0.1m)	7.80224		23.88	2.95	2.0	0.0	23.88	2.95	2.0	0.0	447	10	A	Very Low Risk	10	A	Very Low Risk	
Depth Probe	DP21	T4	51671.0	579096	0.3	Very Shallow (0.01-0.1m)	11.7141		6.31	1.45	2.0	0.0	6.31	1.45	2.0	0.0	244	10	A	Very Low Risk	2.0	B	Low Risk	
Depth Probe	DP22	T4	516716	578976.9	0.1	Very Shallow (0.01-0.1m)	7.80224		23.88	2.95	2.0	0.0	23.88	2.95	2.0	0.0	245	10	A	Very Low Risk	10	A	Very Low Risk	
Depth Probe	DP23	T4	516726	578644	0.65	Very Shallow (0.01-0.1m)	11.8762		3.65	1.01	2.0	0.0	3.65	1.01	2.0	0.0	264	10	A	Very Low Risk	10	A	Very Low Risk	
Depth Probe	DP24	T4	516744	579023	0.2	Very Shallow (0.01-0.1m)	12.26207		7.87	1.93	2.0	0.0	7.87	1.93	2.0	0.0	4	2.0	B	Low Risk	2.0	B	Low Risk	
Depth Probe	DP25	T4	516746	579030	0.2	Very Shallow (0.01-0.1m)	12.26207		7.87	1.93	2.0	0.0	7.87	1.93	2.0	0.0	4	2.0	B	Low Risk	2.0	B	Low Risk	
Depth Probe	DP26	T4	516755	578938.9	0.1	Very Shallow (0.01-0.1m)	12.2451		16.23	1.93	2.0	0.0	16.23	1.93	2.0	0.0	10	1.0	A	Very Low Risk	10	A	Very Low Risk	
Depth Probe	DP27	T4	516760	579050	0.2	Very Shallow (0.01-0.1m)	8.8635		13.33	2.9	2.0	0.0	13.33	2.9	2.0	0.0	302	10	A	Very Low Risk	10	A	Very Low Risk	
Depth Probe	DP28	T4	516774	578648	0.1	Very Shallow (0.01-0.1m)	8.8635	Rock	22.88	2.06	2.0	0.0	22.88	2.06	2.0	0.0	10	1.0	A	Very Low Risk	10	A	Very Low Risk	
Depth Probe	DP29	T4	516781	579066	0.2	Very Shallow (0.01-0.1m)	9.8891		10.02	2.00	2.0	0.0	10.02	2.00	2.0	0.0	317	10	A	Very Low Risk	10	A	Very Low Risk	
Depth Probe	DP30	T4	516786	578998	0.5	Very Shallow (0.01-0.1m)	7.68429	Drain 0.5m	4.98	1.60	2.0	0.0	4.98	1.60	2.0	0.0	10	1.0	A	Very Low Risk	10	A	Very Low Risk	
Depth Probe	DP31	T4	516841	578371	0.3	Very Shallow (0.01-0.1m)	8.9385		6.91	1.93	2.0	0.0	6.91	1.93	2.0	0.0	34.0	10	A	Very Low Risk	10	A	Very Low Risk	
Depth Probe	DP32	T4	516820	578667	0.05	Very Shallow (0.01-0.1m)	8.9389		51.06	2.51	2.0	0.0	51.06	2.51	2.0	0.0	359	10	A	Very Low Risk	10	A	Very Low Risk	
Depth Probe	DP33	T4	516844	578840	0.3	Very Shallow (0.01-0.1m)	10.6882		6.16	1.42	2.0	0.0	6.16	1.42	2.0	0.0	376	10	A	Very Low Risk	10	A	Very Low Risk	
Depth Probe	DP34	T4	516750	578646	0.4	Very Shallow (0.01-0.1m)	12.8074		1.89	2.18	2.0	0.0	1.89	2.18	2.0	0.0	251	10	A	Very Low Risk	2.0	B	Low Risk	
Total	TP05	T4	516781	579123	0.3	Very Shallow (0.01-0.1m)	5.06268		10.99	2.79	2.0	0.0	10.99	2.79	2.0	0.0	301	10	A	Very Low Risk	10	A	Very Low Risk	

Plate 4: Peat Data & Risk Assessment Results – T4



SI Appendix B - Peat & Subsoil Survey Database
Inchamore WF, Co. Cork

Prepared by: SK 07/02/2023
RSK File Ref.: 603675-00.xls

Sample / Test Category	Sample / Test Point ID No.	Association	ITM Easting	ITM Northing	Thickness / Depth of peat	Classification of Thickness / Depth of peat	Slope (Extracted from GDEM)	Note	Scenario A		Scenario B		Scenario A		Scenario B		Scenario A		Scenario B		Scenario A		Scenario B	
									FoS _{PAW} Factor of Safety (FoS) for Peat Stability	FoS _{PAW} Factor of Safety (FoS) for Peat Stability	FoS _{ADJ} Adjusted Factor of Safety (FoS) for Peat Stability	FoS _{ADJ} Adjusted Factor of Safety (FoS) for Peat Stability	RR _{SF} Ranking Potential for Adverse Consequences on Sensitive Receptors	RR _{SF} Ranking Potential for Adverse Consequences on Sensitive Receptors	Distance to Sensitive Receptor	RR _D Risk Ranking Accounting for Distance to Sensitive Receptors	RR _D Risk Ranking Accounting for Distance to Sensitive Receptors	Risk Category	Risk Category					
									CONFIDENT	CONFIDENT	CONFIDENT	CONFIDENT	CONFIDENT	CONFIDENT	CONFIDENT	CONFIDENT	CONFIDENT	CONFIDENT	CONFIDENT	CONFIDENT	CONFIDENT	CONFIDENT	CONFIDENT	CONFIDENT
Depth Probe	DP155	T5	615882.6	679542	0.6	C - Shallow (0.5-2.0m)	3.62814																	
Depth Probe	DP156	T5	616068.6	679535	0.6	A - Very Shallow (0.0-0.5m)	7.20596																	
Depth Probe	DP157	T5	616096.6	679555	0.3	B - Very Shallow (0.0-0.5m)	7.1021																	
Depth Probe	DP158	T5	616194.6	679501	0.1	A - Very Shallow (0.0-0.5m)	8.42544																	
Depth Probe	DP162	T5	616623.6	679874.8	1.2	C - Shallow (0.5-2.0m)	6.54685																	
Total Pk	TP101	T5	616405	679850	0.3	A - Very Shallow (0.0-0.5m)	8.5297																	
Total Pk	TP102	T5	613370	679678	1.0	C - Shallow (0.5-2.0m)	6.22962																	
Total Pk	TP103	T6	613941	679723	0.6	C - Shallow (0.5-2.0m)	6.61429																	

Plate 5: Peat Data & Risk Assessment Results – T5

Plate 6: Peat Data - FoS (ADJ) (B) with Slope (GDEM) presents peat stability risk assessment Factor of Safety (FoS (ADJ) (Scenario B)) results, receptors and associated 50m buffer zones, and slope (GDEM).

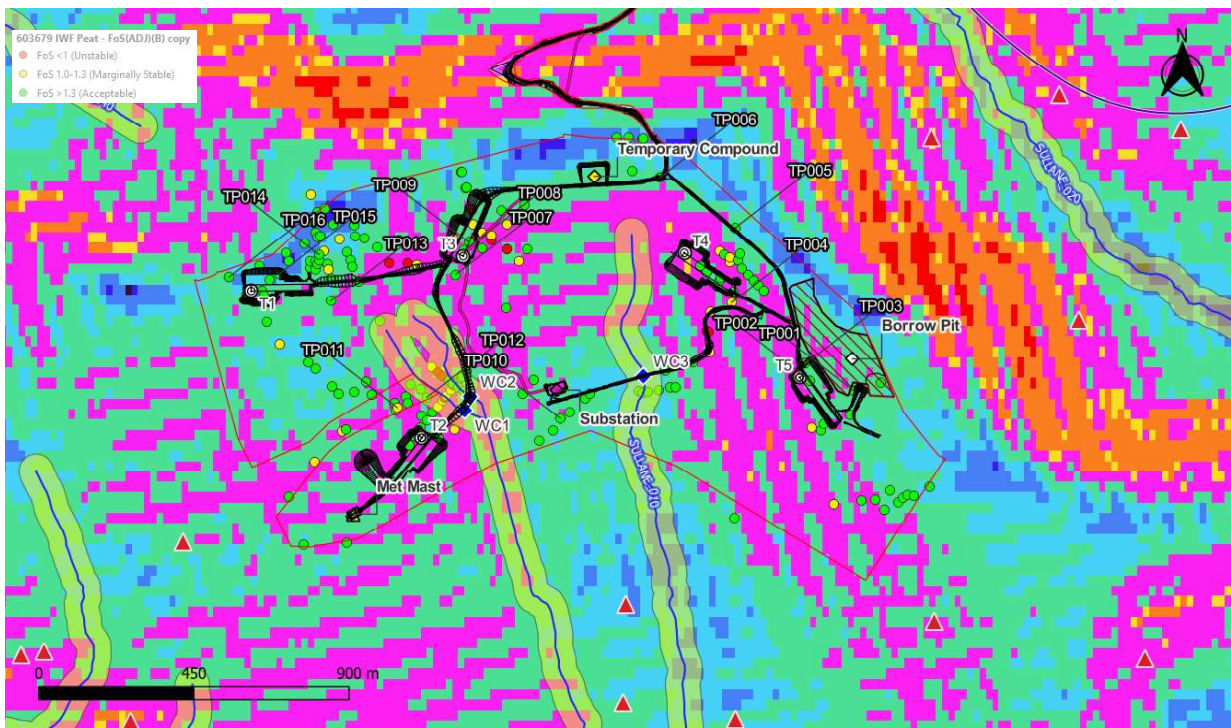


Plate 6: Peat Data - FoS (ADJ) (B) with Slope (GDEM)

4.5 Peat Stability Risk Assessment Interpretation

Table 13: Peat Stability Risk Assessment – Factor of Safety (Adjusted) (Scenario B) at Main Infrastructure Units presents the interpretation of stability risk assessment data in the context of stability, or factor of safety (FoS) (Adjusted, Scenario B) at each significant development infrastructure unit.

Table 13: Peat Stability Risk Assessment – Factor of Safety (Adjusted) (Scenario B) at Main Infrastructure Units

Turbine No. / Unit	FoS _{ADJ} (Factor of Safety adjusted according considering site specific conditions)	Geo-Hazard / Comment (Important to consider when carrying out detailed design and preconstruction planning)
T1	<p>Generally acceptable.</p> <p>Data indicates peat stability is primarily acceptable, with the exception of * pockets of moderately deeper peat (marginally acceptable / unstable at localised scale north of proposed turbine locality).</p>	<p>Localised steep inclines and potential for pockets of deep peat. Residual risk = localised stability issues.</p> <p>Relatively extensive area of deep peat to north / northwest of development footprint at T1. Development footprint avoids this area however vehicular movements must be managed, and this area avoided completely.</p>
T2	<p>Generally acceptable with localised areas of marginally stable FoS, localised areas of unstable peat.</p> <p>Data indicates that peat depth in the area is generally shallow with relatively extensive rock outcrops. Steep inclines in the area are a key driver of unfavourable results.</p>	<p>Localised steep inclines and potential for pockets of deep peat. Residual risk = localised stability issues.</p> <p>Proximity to receptor (river).</p>
T3	<p>Data indicates peat stability is primarily acceptable, marginally acceptable.</p> <p>Some locations on approach (access tracks) possess locally unstable data due to relatively higher localized slope angles, and/or deeper peat however peat depths are shallow.</p>	<p>Localised steep inclines and potential for pockets of deep peat. Residual risk = localised stability issues.</p>
T4	<p>Generally acceptable.</p> <p>Data indicates peat stability is primarily acceptable, with isolated pockets Marginally acceptable.</p>	<p>Localised steep inclines and potential for pockets of deep peat. Residual risk = localised stability issues.</p>
T5	<p>Generally acceptable.</p> <p>Data indicates peat stability is primarily acceptable, with isolated pockets Marginally acceptable.</p>	<p>Localised steep inclines and potential for pockets of deep peat. Residual risk = localised stability issues.</p>

Turbine No. / Unit	FoS_{ADJ} (Factor of Safety adjusted according considering site specific conditions)	Geo-Hazard / Comment (Important to consider when carrying out detailed design and preconstruction planning)
Met Mast	Generally acceptable. Data indicates peat stability is primarily acceptable, with isolated pockets Marginally acceptable.	Localised steep inclines and potential for pockets of deep peat. Residual risk = localised stability issues.
Borrow Pit	Generally acceptable. Data indicates peat stability is primarily acceptable, with isolated pockets Marginally acceptable.	Localised steep inclines and potential for pockets of deep peat. Residual risk = localised stability issues.
Substation	Data indicates peat stability is acceptable. Very Low Risk in terms of Receptors	Potential for localised stability issues.

The following table presents the interpretation of stability risk assessment data in the context of stability, or factor of safety (FoS) in context of receptor type (RR (SF)) and distance to receptor (RR(D)) at each significant development infrastructure unit.

Table 14: Peat Stability Risk Assessment – Factor of Safety (Adjusted) (Scenario B) at Main Infrastructure Units

Turbine No. / Unit	RR(D) (Ranked Risk considering Distance to Sensitive Receptors)	Geo-Hazard / Comment (Important to consider when carrying out detailed design and preconstruction planning)
T1	Very Low to Low Risk	Localised stability and drainage network.
T2	Low to High Risk	Localised stability and proximity to sensitive receptor (river). Minor, localised stability issues have the potential to have significant adverse impacts on receptors.
T3	Very Low to Moderate Risk	Localised stability and drainage network.
T4	Very Low to Moderate Risk	Localised stability and drainage network. Limited data between downstream receptors. Potential for deep pockets of peat but peat depth generally shallow. Max (GDEM) incline = approx. 8 degrees, moderate incline.
T5	Very Low to Low Risk	Localised stability and drainage network.
Met Mast	Very Low to Moderate Risk	Localised stability and drainage network.

Turbine No. / Unit	RR(D) (Ranked Risk considering Distance to Sensitive Receptors)	Geo-Hazard / Comment (Important to consider when carrying out detailed design and preconstruction planning)
Borrow Pit	Very Low to Moderate Risk	Localised stability and drainage network.
Substation	Very Low to Low Risk	Localised stability and drainage network.

4.6 Subsoil Stability Risk Assessment Results

Review of subsoil stability assessment result data and maps as presented in **Appendix I** indicate that the factor of safety is generally acceptable and very low to low stability risk across the site (areas assessed / trial pit locations*) with the exception of minor isolated areas of steeper inclines and deeper till deposits (inferred*).

Summary of risk at the site under varying conditions and scenarios is presented in the following tables.

Table 15: Factor of Safety (Adjusted) at Trial Pit Locations

	Acceptable	Marginally Stable	Unstable
FoS (Adj.) Scenario A	16	0	0
FoS (Adj.) Scenario B	14	2	0

Table 16: Risk Ranking (Distance) at Trial Pit Locations

	Very Low	Low	Moderate	High
RR (Dist.) Scenario A	14	1	1	0
RR (Dist.) Scenario B	13	1	2	0

Based on the inferred conservative values applied to the above stability risk assessment, the factor of safety is highly dependent on cohesive strength, which in turn is highly dependent on hydrogeological characteristics including pore water pressure. **Figure 2** presents potential varying Factors of Safety for subsoils at the Site depending on varying cohesive strength and depths to failure plane.



Figure 2: Correlation Between Factor of Safety, Cohesive Strength and Depth of Subsoils

Observations made during site walkovers include deep deposits of till in the northwestern area of the site immediately north of T1. Iron pan was also observed in trial pits in those areas. The area is also extensively modified in terms of constructed drainage for agricultural and forestry purposes.

Areas with potentially deep till deposits, steep incline (c. >15 degrees), potential for iron pan, and enhanced opportunity for recharge to groundwater are considered to have elevated Moderate to High risk in terms of subsoil soil stability.

Areas of elevated stability risk, even at a localised scale, are considered geo-hazards requiring mitigation. Geo-hazards are presented in **Appendix H**.

4.7 Subsoil Stability Risk Assessment Interpretation

The following table presents the interpretation of stability risk assessment data in the context of stability, or factor of safety (FoS) (Adjusted, Scenario B) at each significant development infrastructure unit.

Table 17: Subsoil Stability Risk Assessment – Risk Ranking (Distance) (Scenario B) at Main Infrastructure Units

Turbine No. / Unit	RR _D (Ranked Risk considering Distance to Sensitive Receptors)	Geo-Hazard / Comment
T1	Low	Localised stability and drainage network.
T2	Low to Moderate	Localised stability and proximity to sensitive receptor (river). Minor, localised stability issues have the potential to have significant adverse impacts on receptors.
T3	Low	Localised stability and drainage network.
T4	Low	Localised stability and drainage network.
T5	Low	Localised stability and drainage network.

Turbine No. / Unit	RR _D (Ranked Risk considering Distance to Sensitive Receptors)	Geo-Hazard / Comment
Met Mast	Low	Localised stability and drainage network.
Borrow Pit	Low	Localised stability and drainage network.
Substation	Low	Localised stability and drainage network.

5. Conclusions

Peat Stability

Peat depth across the site is generally very shallow to shallow with the exception of isolated pockets of moderately deep peat delineated by shallow subsoils and/or bedrock at or near the surface, particularly in the NW of the site. There was no very deep peat observed at the site. There is a relatively extensive area of deep peat north of the proposed location for T1 and the associated access track. The footprint of the Project avoids this area.

The Factor of Safety (Adjusted) (Scenario B i.e., 1m surcharge) at peat probe locations is generally Acceptable throughout the Site with occasional Marginal locations and some Unacceptable localities associated with relatively steeper slopes coupled with relative peat depths.

Marginally Stable Locations, presented in yellow in **Plate 7** above, are concentrated around Site Access tracks and do not overlap with any hardstand areas with the exception of proposed location of T3. Unstable/Unacceptable locations, denoted in red in **Plate 7**, are seen adjacent to the Site Access Tracks to the proposed substation location and T1 as well as the proposed hardstand location of T3.

The Risk Ranking (Distance) Scenario B i.e., 1m surcharge) at peat probe locations is generally Very Low to Low with the exception of Moderate to High-risk point locations, outlined in **Plates 1 - 5** above, mainly associated with close proximity to sensitive receptors (e.g., mapped EPA rivers and artificial draining with direct linkage to rivers). The location of these 'Moderate Risk' to 'High Risk' vary throughout the Site. All proposed turbine hardstand areas are located outside of these elevated risk areas, with the exception of three No. points at T3, Site drainage maps highlight the connection of forestry drains to the Sullane_010.

In summary, through the process of mitigation by design, the Development avoids areas where significant peat or slope stability risk is highest. There remains a residual risk of displacement at a localised scale, which is inherent with all construction / excavation activities particularly when dealing with peat. This is of particular importance to consider when working in close proximity to sensitive receptors, for example; working near, over in surface water features, or when designing drainage networks and the positioning of outfalls.

Subsoil Stability

Subsoils underlying the site are characterized generally as clayey sandy GRAVEL or TILL.

The Factor of Safety (Adjusted) (Scenario B i.e., 1m surcharge) at trial pit locations is generally Acceptable with no exception of marginally stable / unstable point locations.

The Risk Ranking (Distance) Scenario B i.e., 1m surcharge) at trial pit locations is generally Very Low to Low with no exceptions of Moderate or High-risk point locations.

Rock Strength

Bedrock is slightly unweathered.

Bedrock strength at the Site is reported as Weak.

Reuse There is a risk that if used for track surfacing, the trafficked material will gradually degrade, potentially leading to chronic siltation of drainage features or dust depending on meteorological conditions. Therefore, bedrock material arising at the Site will be reused as fill material, Site Access Roads and Turbine Hardstands will be surfaced with a harder rock imported to the Site.



Geo-Hazards

A register of Geo-Hazards is mapped and presented in **Appendix H**.

6. Caveats & Recommendations

The risk of landslides occurring on the proposed site under worst case scenario conditions (Conservative values and Scenario B (+1m)) has been determined to be generally **very low to low** however, the following points should be noted;

- The low risk classification is largely driven by shallow peat depths at sampling points associated with proposed infrastructure locations, and by the undulating nature of the substrate topology, however the potential for deeper areas of peat associated with the Project footprint suggests that soil stability at a highly localized scale may give rise to some difficulty e.g. collapse of side walls in excavations, and subsidence over time under newly installed floating hardstands (on peat), etc. Such potential issues give rise to the need for vigilance during and after the construction phase of the Project and it is recommended that all works are supervised and monitored by a competent person (Geotechnical Engineer) through out the construction phase, and that the site is monitored at a reasonable frequency during the operational phase of the proposed development. The frequency of monitoring during the operational phase will be conducted at a high frequency (e.g. weekly) during the initial months, and will reduce (e.g. monthly) gradually over the following year minimum, or until site conditions are observed to be stable.
- The main infrastructure components such as the turbine hardstand areas avoid very sensitive areas of the site. However, a portion of the proposed access track associated with the proposed watercourse crossings are within 50m of a sensitive receptor (Sullane_010). Peat depths at these locations are shallow however some moderately steep (>8 degrees) to steep (>14 degrees) inclines result in some localised unstable peat data (0.5m peat depth inferred). Unstable peat data in the context of proximity to the downstream receptor (RR(D)) results in a High Risk classification.
- Through EIA, constraint identification and design process, the Project footprint avoids areas of significant unacceptable risk, however this will include all aspects of the Project including; vehicle movements, personell movements, temporary storage, etc. In other words, the Project(including construction activities) will be limited to the Project footprint, and will avoid areas of elevated risk. . Managment of excavation arisings or any bulk material or equipment will consider proximity to these areas or geo-constraints, and developer's or sub-contractors method statement and risk assessments will incorporate this into operational and health and safety mitigation measures.

7. References

Scottish Government (2017) Peat Landslide Hazard and Risk Assessments: Best Practice Guide for Proposed Electricity Generation Developments

N. Boylan, P. Jennings & M. Long (2008) Peat slope failure in Ireland. Quarterly Journal of Engineering Geology and Hydrogeology. Available at https://www.researchgate.net/publication/245379146_Peat_slope_failure_in_Ireland Accessed: 20/08/19.

R. Munro (2004) Dealing with bearing capacity problems on low volume roads constructed on peat. Roadex, Northern Periphery.

GSI Map Viewer. Available at: <http://dcenr.maps.arcgis.com/apps/MapSeries/index.html?appid=a30af518e87a4c0ab2fbde2aaac3c228> Accessed: 20/08/19.

EPA Map Viewer. Available at: <https://gis.epa.ie/EPAMaps/> Accessed: 20/08/19.

EANI River Basin Viewer. Available at <https://appsd.daera-ni.gov.uk/RiverBasinViewer/>. Accessed; 20/08/19.

D. L. Fitzgerald (2007) Estimation of point rainfall frequencies. Irish Meteorological Service Technical Note 61

R. A. Lindsay (2005) Wind Farms and Blanket Peat - The Bog Slide of 16th October 2003 at Derrybrien, Co. Galway, Ireland. University of East London and The Derrybrien Development Cooperative Ltd.

Farrell, E.R., Long, M., Gavin, K. Henry, T.; (2006) 'Chapter 4: Geotechnics of Landslides' In: Creighton, R (eds). Landslides in Ireland. Dublin: Geological Survey of Ireland., pp.23-31

Met Eireann (MET) (2018) 2018, A summer of Heat Waves and Droughts

Met Eireann (MET) (2018) Available Data. Available: <https://www.met.ie> Accessed: 06/08/19

Boylan, N. and Long, M. (2012) Evaluation of peat strength for stability assessments. Geotechnical Engineering Volume 167 Issue GE5, Institution of Civil Engineers (ICE)

Agus F., Hairiah K., Mulyani A. (2011) *Practical Guidelines – Measuring Carbon Stock in Peat Soils*. Bogor, Indonesia: World Agroforestry Centre (ICRAF) Southeast Asia Regional Program, Indonesian Centre for Agricultural Land Resources Research and Development.

Kiley G., Leahy P., McVeigh P., Lewis C., Sottocornola M., Laine A. and Koehler A. (2012) PeatGHG – Survey of GHG Emission and Sink - Potential of Blanket Peatlands. EPA Report No. 228, Environmental Protection Agency (EPA), Ireland.

Renou-Wilson F. and Wilson D. (2014) *Vulnerability Assessment of Peatlands: Exploration of Impacts and Adaptation Options in Relation to Climate Change and Extreme Events (VAPOR)*. EPA Report No. 250, Environmental Protection Agency (EPA), Ireland.

Wilson, S; Bray, R; Cooper, P (CIRIA, 2004) Sustainable Drainage Systems – Hydraulic, structural and water quality advice. CIRIA C609, London, UK.



Geotech Data (ND) Cohesion [Online] Available at: <http://www.geotechdata.info/parameter/cohesion>
Accessed: July 2021

British Standard (BSI) (2010) Code of Practice for Site Investigations (BS 5390:1999 + A2:2010, ISBN
978 0 580 64609 6)



Appendix A

Inchamore Wind Farm
 Inchamore, Co. Cork / Co. Kerry
 App 8.1 - App A1 (01) SI - Peat Depth
 Overview

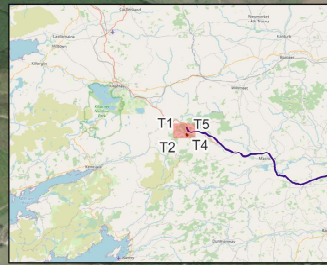
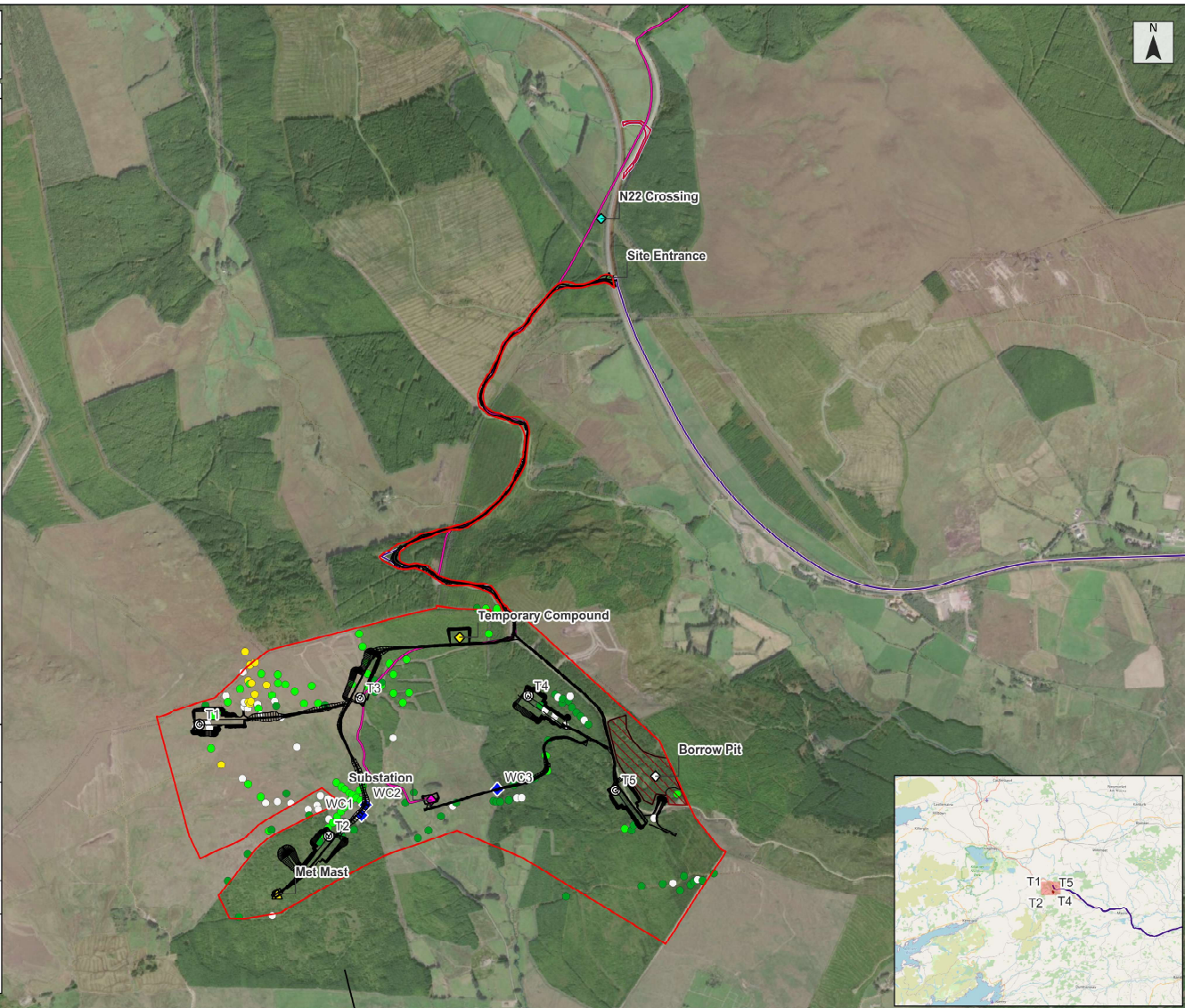
- Legend**
- Development Layout**
- WF
 - Red Line 23
 - 230313 Site Layout
 - Turbine Locations
 - Site Entrances
 - Proposed Met Mast
 - Watercourse Crossings
 - Proposed Borrow Pits
 - Borrow Pit
 - Proposed Temporary Construction Compound
 - Proposed On-Site Substation
- UGC**
- Inchamore Grid Connection Route
 - HDD Crossings
- Delivery**
- Redline-250 Haul Road - 256-Polyline
 - Turbine Delivery Route
 - Redline-250 Haul Road - 256-Polyline
 - Turbine Delivery Route
- Geology**
- 11R8-A7-TWF Peat Depth Probe Data
- 0.0 - 0.1m
 - 0.1 - 0.5m
 - 0.5 - 2.0m
 - 2.0 - 3.5m
 - Geological Linework (100k GSI)
- Base Maps**
- Bing Aerial
 - OpenStreetMap

Project ID: 604162 Inchamore Wind Farm
 Projection: ITM
 Drawn by: Sven K.
 Reviewed by: Sven K.
 Version: 21/09/2022

References/Sources:
 Environmental Protection Agency (EPA)
 Geological Services Ireland (GSI)
 Bing Aerial / Copernicus / Open Street Map / Google Roads
 GDEM Elevation Contours
 Phase 1 (250m Grid Peat Depth) - GreenSource

Note: Data points presented are georeferenced using open source data and/or a handheld GPS. This drawing / map is considered a conceptual model with reasonable accuracy for the purposes of environmental assessment. This drawing should not be relied upon for detailed design purposes.

Scale: 0 0.27 0.54 km



Inchamore Wind Farm
 Inchamore, Co. Cork / Co. Kerry
 App 8.1 - App A2 (01) SI - Peat Depth -
 Tile 1

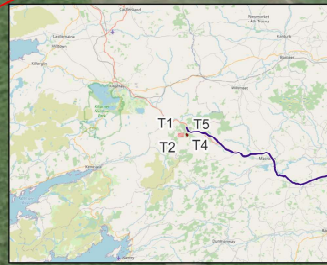
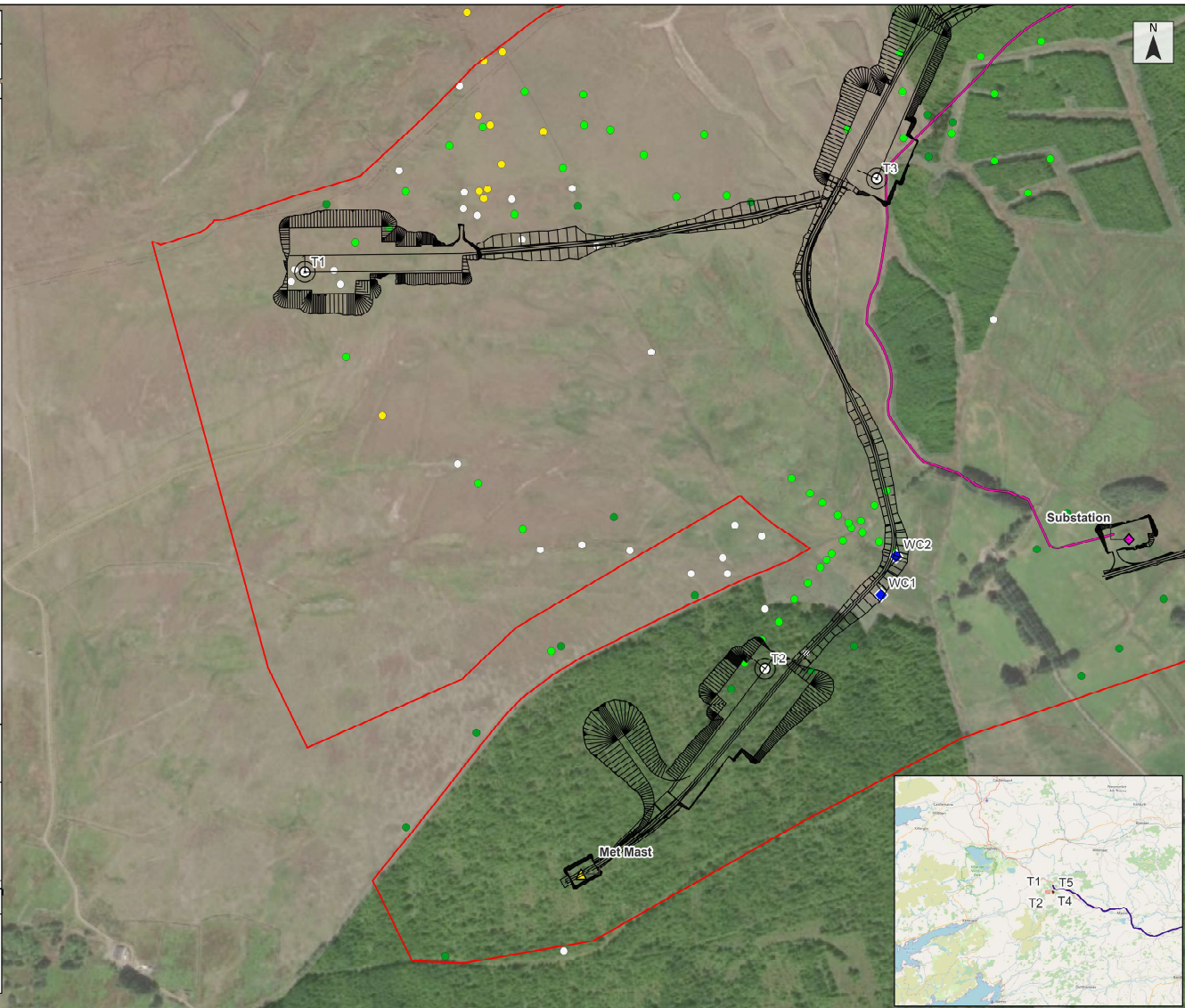
- Legend**
- Development Layout**
- WF
 - Red Line 23
 - 230313 Site Layout
 - Turbine Locations
 - Proposed Met Mast
 - Watercourse Crossings
 - Proposed On-Site Substation
- UGC**
- Inchamore Grid Connection Route
- Geology**
- 3188-A2-WF Peat Depth Probe Data
- 0.0 - 0.1m
 - 0.1 - 0.5m
 - 0.5 - 2.0m
 - 2.0 - 3.5m
- Base Maps**
- Bing Aerial
 - OpenStreetMap

Project ID: 604162 Inchamore Wind Farm
 Projection: ITM
 Drawn by: Sven K.
 Reviewed by: Sven K.
 Version: 21/09/2022

References/Sources:
 Environmental Protection Agency (EPA)
 Geological Services Ireland (GSI)
 Bing Aerial / Coastline / Open Street Map / Google Roads
 GDEM Elevation Contours
 Phase 1 (250m Grid Peat Depth) - Greensource

Note: Data points presented are georeferenced using open source data and/or a handheld GPS. This drawing / map is considered a conceptual model with reasonable accuracy for the purposes of environmental assessment. This drawing should not be relied upon for detailed design purposes.

Scale: 0 0.076 0.152 km



Inchamore Wind Farm
 Inchamore, Co. Cork / Co. Kerry
 App 8.1 - App A2 (01) SI - Peat Depth -
 Tile 2

Legend

Development Layout

- WF
- Red Line 23
- 230313 Site Layout
- Turbine Locations
- Watercourse Crossings
- Proposed Borrow Pits
- Borrow Pit
- Proposed On-Site Substation

UGC

- Inchamore Grid Connection Route

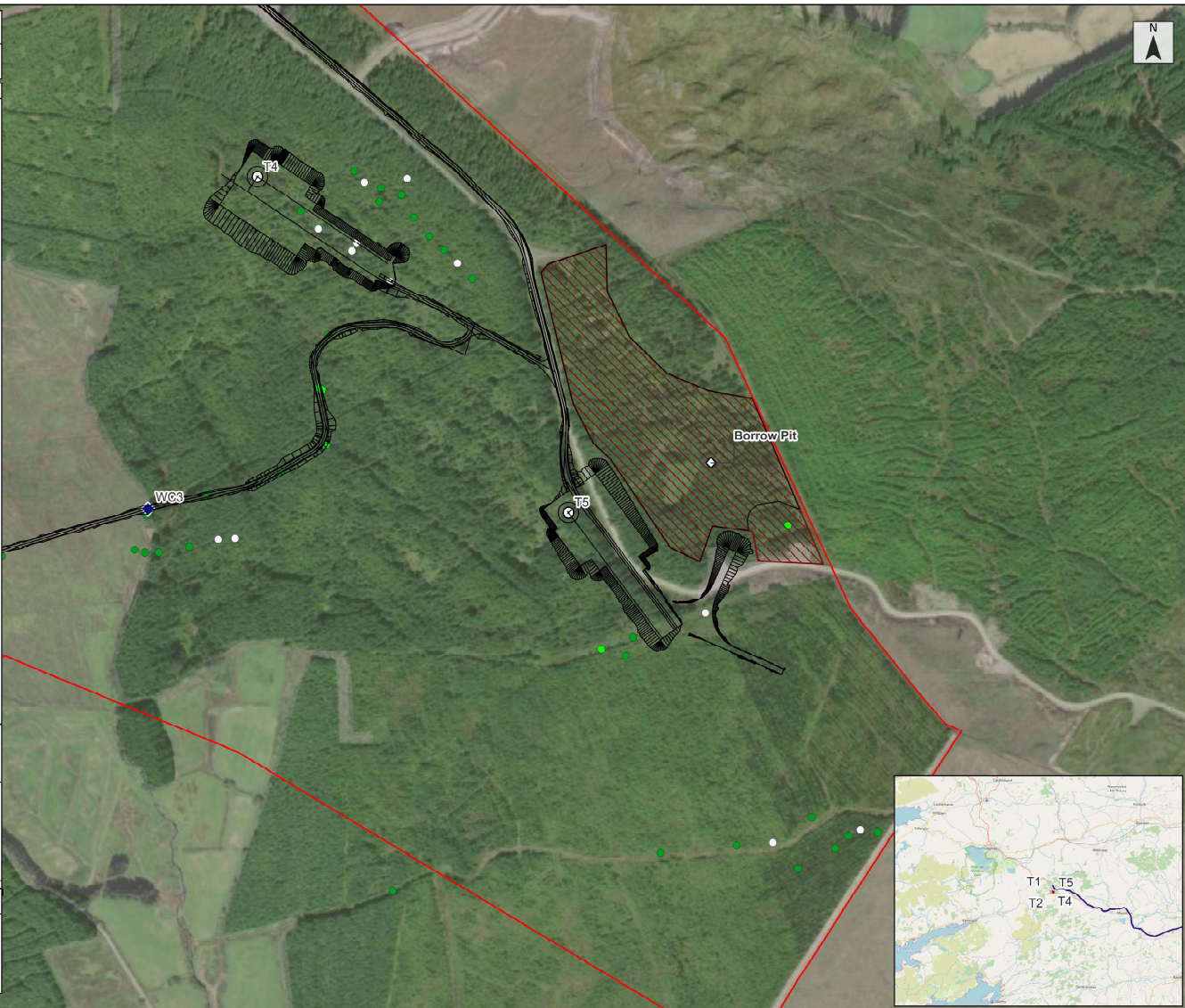
Geology

3188-A2-IWF Peat Depth Probe Data

- 0.0 - 0.1m
- 0.1 - 0.5m
- 0.5 - 2.0m

Base Maps

- Bing Aerial
- OpenStreetMap

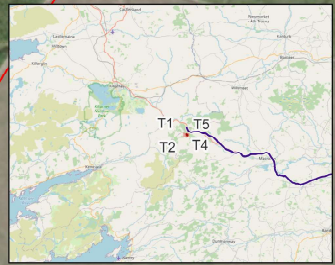


Project ID: 604162 Inchamore Wind Farm
 Projection: ITM
 Drawn by: Sven K.
 Reviewed by: Sven K.
 Version: 21/09/2022

References/Sources:
 Environmental Protection Agency (EPA)
 Geological Services Ireland (GSI)
 Bing Aerial / Copernicus / Open Street Map / Google Roads
 GDEM Elevation Contours
 Phase 1 (250m Grid Peat Depth) - Greensource

Note: Data points presented are georeferenced using open source data and/or a handheld GPS. This drawing / map is considered a conceptual model with reasonable accuracy for the purposes of environmental assessment. This drawing should not be relied upon for detailed design purposes.

Scale: 0 0.076 0.152 km





Appendix B

SJ Appendix B - Peat & Subsoil Survey Database
Inchmare WF, Co. Cork

Peat & Slope Stability Risk Assessment



Prepared by: DK 07/02/2023
RSK File Ref: 60367946L6

Sample / Test Category	Test ID No.	Association	RM Easting	RM Northing	Thickness / Depth of peat	Classification of Thickness / Depth of peat	Slope (Extracted from GDM)	Note	Scenario A		Scenario B		Scenario A		Scenario B		Scenario A		Scenario B		RR _{SP}	RR _{SP}	RR _D	RR _D	Risk Category	Risk Category
									Un drained Strength (kPa)	BU Unit weight (kN/m ³)	Factor of Safety (FoS) _{RAW}	Factor of Safety (FoS) _{RAW}	FoS _{ADU}	FoS _{ADU}	RR _{SP}	RR _{SP}	RR _D	RR _D								
Depth	Angle	W	H	c'	γ	z	α	σ _v	σ _{v'}	τ	τ _{ult}	F _{0.1}	F _{0.2}	F _{0.5}	F _{1.0}	F _{1.5}	F _{2.0}	F _{2.5}	F _{3.0}	F _{3.5}	F _{4.0}	F _{4.5}	F _{5.0}	F _{5.5}	F _{6.0}	
Depth Probe	09190	SETE	51363.144	57669.427	0.3	Very Thin (<0.5m)	5.5346 REFERRED, VISIBLE	0.2980	0.0666	3.0	11	0.300	1.38	1.156	0.98	2.58	11.95	0.98	2.58	11.95	0.98	2.58	11.95	0.98	2.58	11.95

DFY	DFZ	DFY	DFZ	DFY	DFZ	DFY	DFZ	DFY	DFZ	DFY	DFZ	DFY	DFZ	DFY	DFZ	DFY	DFZ	DFY	DFZ	DFY	DFZ	DFY	DFZ	DFY	DFZ
131.80	0.300	128.338	0.300	134	0.300	147	0.300	150	0.300	153	0.300	156	0.300	159	0.300	162	0.300	165	0.300	168	0.300	171	0.300	174	0.300

FCS	FOS	FOS _{ADU}	RR _{SP}	RR _{SP}	RR _D	RR _D	Risk Category	Risk Category
0.5	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0

FCS	FOS	FOS _{ADU}	RR _{SP}	RR _{SP}	RR _D	RR _D	Risk Category	Risk Category
0.5	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0



Appendix C

Inchamore Wind Farm
 Inchamore, Co. Cork / Co. Kerry
 App 8.1 - App C- 3188-A2 (01) IWF SI -
 TP DIH Locations

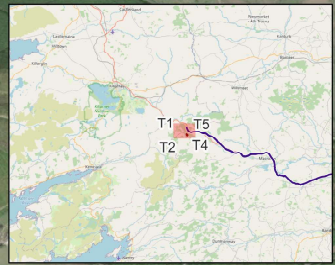
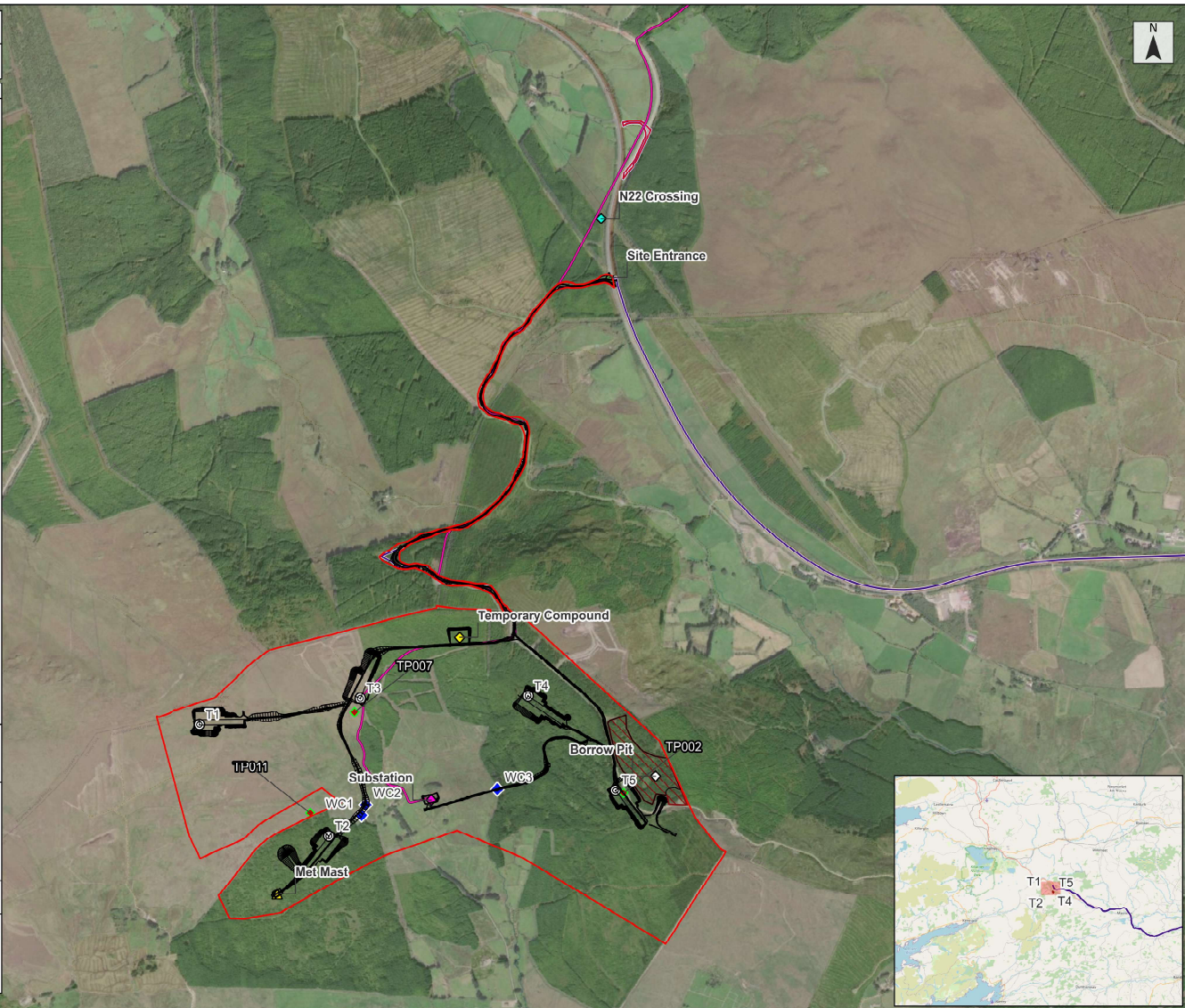
- Legend**
- Development Layout**
- WF
 - Red Line 23
 - 230313 Site Layout
 - Turbine Locations
 - Site Entrances
 - Proposed Met Mast
 - Watercourse Crossings
 - Proposed Borrow Pits
 - Borrow Pit
 - Proposed Temporary Construction Compound
 - Proposed On-Site Substation
- UGC**
- Inchamore Grid Connection Route
 - HDD Crossings
- Delivery**
- Redline-250 Haul Road - 256-Polyline
 - Turbine Delivery Route
 - Redline-250 Haul Road - 256-Polyline
 - Turbine Delivery Route
- Geology**
- 3188-A2-IWF SI Trial Pit Data
- Yes, Iron Pan Present
- Base Maps**
- Bing Aerial
 - Openstreetmap

Project ID: 604162 Inchamore Wind Farm
 Projection: ITM
 Drawn by: Sven K.
 Reviewed by: Sven K.
 Version: 21/09/2022

References/Sources:
 Environmental Protection Agency (EPA)
 Geological Services Ireland (GSI)
 Bing Aerial / Copernicus / Open Street Map / Google Roads
 GDEM Elevation Contours
 Phase 1 (250m Grid Point Depth) - GreenSource

Note: Data points presented are georeferenced using open source data and/or a handheld GPS. This drawing / map is considered a conceptual model with reasonable accuracy for the purposes of environmental assessment. This drawing should not be relied upon for detailed design purposes.

Scale: 0 0.27 0.54 km





Appendix D




Inchamore WF, Co. Cork

SI Trial Pit Logs

Borehole/ Trial Pit Design & Completion	Soil (S) / Water (W) / Vapour (V) Sampling					Depth in metres below ground level, also (maODM) & [Thickness]	Geology (graphical log)	INVESTIGATION POINT LOG NUMBER	
	Sample number & interval (mbGL) (Sample 10 kg minimum) Red line = Single channel sample (from field) Blue line = Composite sample (generated in office or lab) Green line = Grab sample (acquired on site)	Non-Natural Ground Percentage (see * below)	PID (ppm) Bagged sample (BS); Trial Pit Wall (TPW); Soil Core (SC); BH Artisings (BHA); Trial Pit Clumps (TPC)	Odour strength & description (none, weak, moderate, strong)	Groundwater occurrence (See legend for symbols used for dry, damp and wet)			Client, Project, Location	TP001
N/A		N/A	N/A			0.0 - 0.5	PEAT/PEATY SOIL. Medium Brown	N	
						0.5 - 0.8	Sandy Gravelly CLAY w/ Cobbles. Medium Brown	N	
						0.8 - 1.0	Boulders / Weathered Bedrock		
						1.0 - 1.5	EOH – Weathered Bedrock / Boulders	N	
						1.5 - 2.0			
						2.0 - 2.5			
						2.5 - 3.0			
						3.0 - 3.5			
						3.5 - 4.0			

* **Non-natural material %s with total % in ()**
NON-DEGRADABLE % (ND): 1 = Brick, 2 = Concrete, 3 = Glass, 4 = Ceramic tiles, 5 = ACMs (asbestos containing materials such as roof tiles, piping). 6 = Blue Bangor slate.
DEGRADABLE % (D): 7 = Plastic, 8 = Metal, 9 = Wood / Organic / Leaves / Twigs / Peat, 10 = Ash & Clinker, 11 = Charcoal, 12 = Tarmacadam, 13 = Leather, 14 = Coal Tar


** 1-From hand held GPS, 2-Estimated from google maps or 3-Surveyed with theodolite.

A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit	B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder	C COLOUR - Brown (LB, MB, DB) - Grey (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange	D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff	E LAYER ID, RECOVERY & STONE % recovery % >10mm stone	F NN or N
 Write additional help notes on macropores, mottling etc as space allows		F Interpretation NN = Non-natural ground (fill / made up ground / disturbed natural); N = Natural ground			

Borehole/ Trial Pit Design & Completion	Soil (S) / Water (W) / Vapour (V) Sampling					Depth in metres below ground level, also (maODM) & [Thickness]	Geology (graphical log)	INVESTIGATION POINT LOG NUMBER		
	Sample number & interval (mbGL) (Sample 10 kg minimum)	Non-Natural Ground Percentage (see * below)	PID (ppm)	Odour strength & description (none, weak, moderate, strong)	Groundwater occurrence (See legend for symbols used for dry, damp and wet)			Client, Project, Location	TP002	Geological description
N/A		N/A	N/A			0.5	CLAY. Grey Brown			N
						1.0	PEAT/PEATY SOIL. Medium Brown			N
						1.5	Sandy Gravelly CLAY. Grey Brown			N
						2.0	Sandy Gravelly Cobbly CLAY w/ Boulders. Purplish Grey			N
						2.5	Bigger Boulders			
						3.0	EOH – Obstruction (Boulders)			
						3.5				
						4.0				

* **Non-natural material %s with total % in ()**
NON-DEGRADABLE % (ND): 1 = Brick, 2 = Concrete, 3 = Glass, 4 = Ceramic tiles, 5 = ACMs (asbestos containing materials such as roof tiles, piping). 6 = Blue Bangor slate.
DEGRADABLE % (D): 7 = Plastic, 8 = Metal, 9 = Wood / Organic / Leaves / Twigs / Peat, 10 = Ash & Clinker, 11 = Charcoal, 12 = Tarmacadam, 13 = Leather, 14 = Coal Tar

** 1-From hand held GPS, 2-Estimated from google maps or 3-Surveyed with theodolite.

A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit	B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder	C COLOUR - Brown (LB, MB, DB) - Grey (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange	D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff	E LAYER ID, RECOVERY & STONE % recovery % >10mm stone	F NN or N
					F Interpretation NN = Non-natural ground (fill / made up ground / disturbed natural); N = Natural ground)

Write additional help notes on macropores, mottling etc as space allows

Borehole/ Trial Pit Design & Completion	Soil (S) / Water (W) / Vapour (V) Sampling					Depth in metres below ground level, also (maODM) & [Thickness]	Geology (graphical log)	INVESTIGATION POINT LOG NUMBER	
	Sample number & interval (mbGL) (Sample 10 kg minimum) Red line = Single channel sample (from field) Blue line = Composite sample (generated in office or lab) Green line = Grab sample (acquired on site)	Non-Natural Ground Percentage	PID (ppm) Bagged sample (BS); Trial Pit Wall (TPW); Soil Core (SC); BH Arisings (BHA); Trial Pit Clumps (TPC)	Odour strength & description (none, weak, moderate, strong)	Groundwater occurrence (See legend for symbols used for dry, damp and wet)			Client, Project, Location	TP003
N/A		N/A	N/A			0.0 - 0.5	PEAT/PEATY SOIL. Dark Brown; Mixed/Disturbed		N
						0.5 - 1.0	Sandy Gravelly CLAY. Brown Grey		N
						1.0 - 3.0	Sandy Gravelly CLAY w/ Cobbles and Boulders. Blue Grey / Purple Grey. Bigger Boulders		N
						3.0 - 4.0	EOH		

* Unreliable data. Indication only.

** From hand held GPS

A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit	B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder	C COLOUR (LB, MB, DB) - Brown (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange	D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff	E LAYER ID, RECOVERY & STONE % recovery % >10mm stone	F NN or N
--	--	---	---	---	---------------------

RSK

Write additional help notes on macropores, mottling etc as space allows

F Interpretation
NN = Non-natural ground (fill / made up ground / disturbed natural);
N = Natural ground

Borehole/ Trial Pit Design & Completion	Soil (S) / Water (W) / Vapour (V) Sampling					Depth in metres below ground level, also (maODM) & [Thickness]	Geology (graphical log)	INVESTIGATION POINT LOG NUMBER	
	Sample number & interval (mbGL) (Sample 10 kg minimum) Red line = Single channel sample (from field) Blue line = Composite sample (generated in office or lab) Green line = Grab sample (acquired on site)	Non-Natural Ground Percentage	PID (ppm) Bagged sample (BS); Trial Pit Wall (TPW); Soil Core (SC); BH Artisings (BHA); Trial Pit Clumps (TPC)	Odour strength & description (none, weak, moderate, strong)	Groundwater occurrence (See legend for symbols used for dry, damp and wet)			Client, Project, Location	TP004
N/A		N/A	N/A					PEAT/PEATY SOIL. Dark Brown	N
					0.5		Sandy CLAY. Medium Brown	N	
					1.0		Sandy CLAY. Grey	N	
					1.5				
					2.0				
					2.5				
					3.0				
					3.5				
					4.0				

* Unreliable data. Indication only.
 ** From hand held GPS

A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit	B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder	C COLOUR - Brown (LB, MB, DB) - Grey (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange	D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff	E LAYER ID, RECOVERY & STONE % recovery % >10mm stone	F NN or N
--	--	---	---	---	---------------------

RSK

Write additional help notes on macropores, mottling etc as space allows


F Interpretation
 NN = Non-natural ground (fill / made up ground / disturbed natural);
 N = Natural ground)

Borehole/ Trial Pit Design & Completion	Soil (S) / Water (W) / Vapour (V) Sampling					Depth in metres below ground level, also (maODM) & [Thickness]	Geology (graphical log)	INVESTIGATION POINT LOG NUMBER							
	Sample number & interval (mbGL) (Sample 10 kg minimum) Red line = Single channel sample (from field) Blue line = Composite sample (generated in office or lab) Green line = Grab sample (acquired on site)	Non-Natural Ground Percentage	PID (ppm) Bagged sample (BS); Trial Pit Wall (TPW); Soil Core (SC); BH Artisings (BHA); Trial Pit Clumps (TPC)	Odour strength & description (none, weak, moderate, strong)	Groundwater occurrence (See legend for symbols used for dry, damp and wet)			Client, Project, Location	TP005	Geological description					
N/A		N/A	N/A			0.0 - 0.1		PEAT/PEATY SOIL. Dark Brown	N						
						0.1 - 4.0		Sandy Gravelly CLAY w/ boulders. Medium Brown	N						
						4.0 - 4.1		EOH - Boulders							
<p>* Unreliable data. Indication only.</p> <p>** From hand held GPS</p>							<table border="1"> <tr> <td>A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit</td> <td>B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder</td> <td>C COLOUR - Brown (LB, MB, DB) - Grey (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange</td> <td>D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff</td> <td>E LAYER ID, RECOVERY & STONE % recovery % >10mm stone</td> <td>F NN or N</td> </tr> </table> <p>RSK</p> <p>Write additional help notes on macropores, mottling etc as space allows</p> <p>F Interpretation NN = Non-natural ground (fill / made up ground / disturbed natural); N = Natural ground</p>			A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit	B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder	C COLOUR - Brown (LB, MB, DB) - Grey (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange	D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff	E LAYER ID, RECOVERY & STONE % recovery % >10mm stone	F NN or N
A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit	B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder	C COLOUR - Brown (LB, MB, DB) - Grey (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange	D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff	E LAYER ID, RECOVERY & STONE % recovery % >10mm stone	F NN or N										

Borehole/ Trial Pit Design & Completion	Soil (S) / Water (W) / Vapour (V) Sampling					Depth in metres below ground level, also (maODM) & [Thickness]	Geology (graphical log)	INVESTIGATION POINT LOG NUMBER	
	Sample number & interval (mbGL) (Sample 10 kg minimum) Red line = Single channel sample (from field) Blue line = Composite sample (generated in office or lab) Green line = Grab sample (acquired on site)	Non-Natural Ground Percentage	PID (ppm) Bagged sample (BS); Trial Pit Wall (TPW); Soil Core (SC); BH Artisings (BHA); Trial Pit Clumps (TPC)	Odour strength & description (none, weak, moderate, strong)	Groundwater occurrence (See legend for symbols used for dry, damp and wet)			Client, Project, Location	TP007
N/A		N/A	N/A			0.0 - 0.1	PEAT/PEATY SOIL. Dark Brown		N
						0.1 - 0.6	Sandy Gravelly CLAY. Medium Brown		N
						0.6 - 1.0	Sandy Gravelly CLAY with cobbles. Blue grey		N
						1.0 - 1.2	Iron stain		
						1.2 - 1.5	Iron stain		
						1.5 - 2.0	Iron stain		
						2.0 - 3.2	EOH – Weathered Bedrock / Boulders		

* Unreliable data. Indication only.

** From hand held GPS

A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit	B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder	C COLOUR - Brown (LB, MB, DB) - Grey (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange	D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff	E LAYER ID, RECOVERY & STONE % recovery % >10mm stone	F NN or N
					F Interpretation NN = Non-natural ground (fill / made up ground / disturbed natural); N = Natural ground)

Write additional help notes on macropores, mottling etc as space allows

Borehole/ Trial Pit Design & Completion	Soil (S) / Water (W) / Vapour (V) Sampling					Depth in metres below ground level, also (maODM) & [Thickness]	Geology (graphical log)	INVESTIGATION POINT LOG NUMBER	
	Sample number & interval (mbGL) (Sample 10 kg minimum) Red line = Single channel sample (from field) Blue line = Composite sample (generated in office or lab) Green line = Grab sample (acquired on site)	Non-Natural Ground Percentage	PID (ppm) Bagged sample (BS); Trial Pit Wall (TPW); Soil Core (SC); BH Artisings (BHA); Trial Pit Clumps (TPC)	Odour strength & description (none, weak, moderate, strong)	Groundwater occurrence (See legend for symbols used for dry, damp and wet)			Client, Project, Location	TP008
N/A		N/A	N/A				PEAT/PEATY SOIL. Dark Brown	N	
				0.5		Sandy Gravelly CLAY w/ cobbles.	N	
					1.0			N	
					1.5			N	
					2.0			N	
					2.5			N	
					3.0		EOH - Weathered Bedrock / Boulders		
					3.5				
					4.0				

* Unreliable data. Indication only.

** From hand held GPS

A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit	B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder	C COLOUR - Brown (LB, MB, DB) - Grey (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange	D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff	E LAYER ID, RECOVERY & STONE % recovery % >10mm stone	F NN or N
					F Interpretation NN = Non-natural ground (fill / made up ground / disturbed natural); N = Natural ground

Write additional help notes on macropores, mottling etc as space allows

Borehole/ Trial Pit Design & Completion	Soil (S) / Water (W) / Vapour (V) Sampling					Depth in metres below ground level, also (maODM) & [Thickness]	Geology (graphical log)	INVESTIGATION POINT LOG NUMBER		
	Sample number & interval (mbGL) (Sample 10 kg minimum) Red line = Single channel sample (from field) Blue line = Composite sample (generated in office or lab) Green line = Grab sample (acquired on site)	Non-Natural Ground Percentage	PID (ppm) Bagged sample (BS); Trial Pit Wall (TPW); Soil Core (SC); BH Artisings (BHA); Trial Pit Clumps (TPC)	Odour strength & description (none, weak, moderate, strong)	Groundwater occurrence (See legend for symbols used for dry, damp and wet)			Client, Project, Location	TP009	Geological description
N/A		N/A	N/A				PEAT/PEATY SOIL. Dark Brown Sandy Gravelly CLAY w/ cobbles. Medium Brown Sandy Gravelly CLAY w/ cobbles and boulders. Medium Brown Bigger Boulders EOH	N	N	N

* Unreliable data. Indication only.

** From hand held GPS

A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit	B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder	C COLOUR - Brown (LB, MB, DB) - Grey (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange	D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff	E LAYER ID, RECOVERY & STONE % recovery % >10mm stone	F NN or N
					F Interpretation NN = Non-natural ground (fill / made up ground / disturbed natural); N = Natural ground

Write additional help notes on macropores, mottling etc as space allows

Borehole/ Trial Pit Design & Completion	Soil (S) / Water (W) / Vapour (V) Sampling					Depth in metres below ground level, also (maODM) & [Thickness]	Geology (graphical log)	INVESTIGATION POINT LOG NUMBER		
	Sample number & interval (mbGL) (Sample 10 kg minimum) Red line = Single channel sample (from field) Blue line = Composite sample (generated in office or lab) Green line = Grab sample (acquired on site)	Non-Natural Ground Percentage	PID (ppm) Bagged sample (BS); Trial Pit Wall (TPW); Soil Core (SC); BH Artisings (BHA); Trial Pit Clumps (TPC)	Odour strength & description (none, weak, moderate, strong)	Groundwater occurrence (See legend for symbols used for dry, damp and wet)			Client, Project, Location	TP010	Geological description
N/A		N/A	N/A				<p>TOPSOIL</p> <p>Sandy Gravelly CLAY. Brown</p> <p>PEAT/PEATY SOIL. Dark Brown</p> <p>Sandy Gravelly CLAY. Brown</p> <p>Sandy Gravelly CLAY. Blue Grey</p> <p>Big Boulder</p> <p>EOH</p>	N	N	

* Unreliable data. Indication only.

** From hand held GPS

A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit	B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder	C COLOUR - Brown (LB, MB, DB) - Grey (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange	D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff	E LAYER ID, RECOVERY & STONE % recovery % >10mm stone	F NN or N
<p>RSK</p> <p>Write additional help notes on macropores, mottling etc as space allows</p> <p>F Interpretation NN = Non-natural ground (fill / made up ground / disturbed natural); N = Natural ground)</p>					

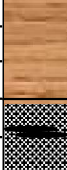

Borehole/ Trial Pit Design & Completion	Soil (S) / Water (W) / Vapour (V) Sampling					Depth in metres below ground level, also (maODM) & [Thickness]	Geology (graphical log)	INVESTIGATION POINT LOG NUMBER	
	Sample number & interval (mbGL) (Sample 10 kg minimum) Red line = Single channel sample (from field) Blue line = Composite sample (generated in office or lab) Green line = Grab sample (acquired on site)	Non-Natural Ground Percentage	PID (ppm) Bagged sample (BS); Trial Pit Wall (TPW); Soil Core (SC); BH Arisings (BHA); Trial Pit Clumps (TPC)	Odour strength & description (none, weak, moderate, strong)	Groundwater occurrence (See legend for symbols used for dry, damp and wet)			Client, Project, Location	TP011
N/A		N/A	N/A				PEAT/PEATY SOIL. Dark Brown	N	
							Very clayey very sandy GRAVEL.	N	
							EOH – Weathered Bedrock		

* Unreliable data. Indication only.

** From hand held GPS


A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit	B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder	C COLOUR - Brown (LB, MB, DB) - Grey (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange	D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff	E LAYER ID, RECOVERY & STONE % recovery % >10mm stone	F NN or N
					F Interpretation NN = Non-natural ground (fill / made up ground / disturbed natural); N = Natural ground

Write additional help notes on macropores, mottling etc as space allows

Borehole/ Trial Pit Design & Completion	Soil (S) / Water (W) / Vapour (V) Sampling					Depth in metres below ground level, also (maODM) & [Thickness]	Geology (graphical log)	INVESTIGATION POINT LOG NUMBER	
	Sample number & interval (mbGL) (Sample 10 kg minimum) Red line = Single channel sample (from field) Blue line = Composite sample (generated in office or lab) Green line = Grab sample (acquired on site)	Non-Natural Ground Percentage	PID (ppm) Bagged sample (BS); Trial Pit Wall (TPW); Soil Core (SC); BH Artisings (BHA); Trial Pit Clumps (TPC)	Odour strength & description (none, weak, moderate, strong)	Groundwater occurrence (See legend for symbols used for dry, damp and wet)			Client, Project, Location	TP012
N/A		N/A	N/A			0.5		PEAT/PEATY SOIL. Dark Brown	N
						0.5		Weathered Bedrock	N
						1.0		EOH – Weathered Bedrock	
						1.5			
						2.0			
						2.5			
						3.0			
						3.5			
						4.0			

* Unreliable data. Indication only.

** From hand held GPS

A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit	B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder	C COLOUR - Brown (LB, MB, DB) - Grey (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange	D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff	E LAYER ID, RECOVERY & STONE % recovery % >10mm stone	F NN or N
					F Interpretation NN = Non-natural ground (fill / made up ground / disturbed natural); N = Natural ground

Write additional help notes on macropores, mottling etc as space allows

Borehole/ Trial Pit Design & Completion	Soil (S) / Water (W) / Vapour (V) Sampling					Depth in metres below ground level, also (maODM) & [Thickness]	Geology (graphical log)	INVESTIGATION POINT LOG NUMBER	
	Sample number & interval (mbGL) (Sample 10 kg minimum) Red line = Single channel sample (from field) Blue line = Composite sample (generated in office or lab) Green line = Grab sample (acquired on site)	Non-Natural Ground Percentage	PID (ppm) Bagged sample (BS); Trial Pit Wall (TPW); Soil Core (SC); BH Artisings (BHA); Trial Pit Clumps (TPC)	Odour strength & description (none, weak, moderate, strong)	Groundwater occurrence (See legend for symbols used for dry, damp and wet)			Client, Project, Location	TP013
N/A		N/A	N/A				PEAT/PEATY SOIL. Dark Brown	N	
							Sandy Gravelly CLAY	N	
							Sandy Gravelly CLAY w/ cobbles and boulder. Blue grey.		
							EOH – Boulders		

* Unreliable data. Indication only.

** From hand held GPS

A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit	B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder	C COLOUR - Brown (LB, MB, DB) - Grey (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange	D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff	E LAYER ID, RECOVERY & STONE % recovery % >10mm stone	F NN or N
					F Interpretation NN = Non-natural ground (fill / made up ground / disturbed natural); N = Natural ground

Write additional help notes on macropores, mottling etc as space allows

Borehole/ Trial Pit Design & Completion	Soil (S) / Water (W) / Vapour (V) Sampling					Depth in metres below ground level, also (maODM) & [Thickness]	Geology (graphical log)	INVESTIGATION POINT LOG NUMBER		
	Sample number & interval (mbGL) (Sample 10 kg minimum) Red line = Single channel sample (from field) Blue line = Composite sample (generated in office or lab) Green line = Grab sample (acquired on site)	Non-Natural Ground Percentage	PID (ppm) Bagged sample (BS); Trial Pit Wall (TPW); Soil Core (SC); BH Artisings (BHA); Trial Pit Clumps (TPC)	Odour strength & description (none, weak, moderate, strong)	Groundwater occurrence (See legend for symbols used for dry, damp and wet)			Client, Project, Location	TP014	Geological description
N/A		N/A	N/A			0.0		PEAT/PEATY SOIL. Dark Brown		N
						0.5		TP abandoned, deep peat encountered, probe point <5m from TP = 2.0mbGL. See peat probing data for area.		
						1.0				
						1.5				
						2.0				
						2.5				
						3.0				
						3.5				
						4.0				


* Unreliable data. Indication only.
 ** From hand held GPS



A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit	B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder	C COLOUR (LB, MB, DB) - Grey (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange	D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff	E LAYER ID, RECOVERY & STONE % recovery % >10mm stone	F NN or N
RSK					
Write additional help notes on macropores, mottling etc as space allows					
F Interpretation NN = Non-natural ground (fill / made up ground / disturbed natural); N = Natural ground)					

Borehole/ Trial Pit Design & Completion	Soil (S) / Water (W) / Vapour (V) Sampling					Depth in metres below ground level, also (maODM) & [Thickness]	Geology (graphical log)	INVESTIGATION POINT LOG NUMBER	
	Sample number & interval (mbGL) (Sample 10 kg minimum) Red line = Single channel sample (from field) Blue line = Composite sample (generated in office or lab) Green line = Grab sample (acquired on site)	Non-Natural Ground Percentage	PID (ppm) Bagged sample (BS); Trial Pit Wall (TPW); Soil Core (SC); BH Artisings (BHA); Trial Pit Clumps (TPC)	Odour strength & description (none, weak, moderate, strong)	Groundwater occurrence (See legend for symbols used for dry, damp and wet)			Client, Project, Location	TP015
N/A		N/A	N/A			0.0 - 0.2	PEAT/PEATY SOIL. Dark Brown	N	JOD (Coillte), Inchamore WF, Cork
						0.2 - 0.5	Sandy Gravelly Cobbly CLAY, Brown	N	Minerex work item
						0.5 - 4.0	EOH. Bedrock		A2
									Page No.
									1 of 1
									Date & time drilled / formed:
									03/06/2021
									Logged by (drawn by) [checked by]:
									SK
									Drilling / Trial pitting co. & equipment
									Excavator
									Doc. Ref.
									(File Ref. 3188-A2-024; 603679 App D)
									Irish Transverse Mercator (ITM)**
									512439, 578989
									Natural / Made

* Unreliable data. Indication only.


** From hand held GPS

A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit	B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder	C COLOUR - Brown (LB, MB, DB) - Grey (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange	D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff	E LAYER ID, RECOVERY & STONE % recovery % >10mm stone	F NN or N
					F Interpretation
Write additional help notes on macropores, mottling etc as space allows					NN = Non-natural ground (fill / made up ground / disturbed natural); N = Natural ground

Borehole/ Trial Pit Design & Completion	Soil (S) / Water (W) / Vapour (V) Sampling					Depth in metres below ground level, also (maODM) & [Thickness]	Geology (graphical log)	INVESTIGATION POINT LOG NUMBER	
	Sample number & interval (mbGL) (Sample 10 kg minimum) Red line = Single channel sample (from field) Blue line = Composite sample (generated in office or lab) Green line = Grab sample (acquired on site)	Non-Natural Ground Percentage	PID (ppm) Bagged sample (BS); Trial Pit Wall (TPW); Soil Core (SC); BH Artisings (BHA); Trial Pit Clumps (TPC)	Odour strength & description (none, weak, moderate, strong)	Groundwater occurrence (See legend for symbols used for dry, damp and wet)			Client, Project, Location	TP016
N/A		N/A	N/A			0.0 - 0.1	 PEAT/PEATY SOIL. Dark Brown	N	JOD (Coillte), Inchamore WF , Cork Minerex work item: A2 Page No.: 1 of 1 Date & time drilled / formed: 03/06/2021 Logged by (drawn by) [checked by]: SK Drilling / Trial pitting co. & equipment: Excavator Doc. Ref.: (File Ref. 3188-A2-024; 603679 App D) Irish Transverse Mercator (ITM)**: 512293, 578980 Geological description:
						0.1 - 0.5	 Sandy Gravelly Cobbly CLAY, Brown	N	
						0.5 - 4.0	EOH. Bedrock.		

* Unreliable data. Indication only.

** From hand held GPS

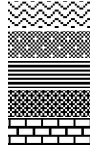
A DOMINANT GEOLOGICAL COMPONENT Clay, Silt, Sand, Gravel, Cobble, Boulder deposit	B NON-DOMINANT GEOLOGICAL COMPONENT Clay - Silt - Sand Gravel - Cobble - Boulder	C COLOUR - Brown (LB, MB, DB) - Grey (LG, MG, DG) - Mustard - Beige (tan) - Olive - Mottled - Orange	D STIFFNESS VST - V. Soft ST = Soft F = Firm S = Stiff VS = V. Stiff	E LAYER ID, RECOVERY & STONE % recovery % >10mm stone	F NN or N
					F Interpretation NN = Non-natural ground (fill / made up ground / disturbed natural); N = Natural ground

Write additional help notes on macropores, mottling etc as space allows

GENERAL LEGEND, ABBREVIATIONS AND INSTALLATION DETAILS

BEDROCK

- Metamorphic bedrock
- Igneous bedrock
- Mudstone / Shale bedrock
- Siltstone / Sandstone bedrock
- Limestone bedrock



COLOUR

- Brown** (Light, medium, dark)
- Grey** (Light, medium, dark)
- Mustard**
- Beige (tan)**
- Olive**
- Mottled**
- Orange**

GRAIN SIZE (Soil)

- Clay** (% of) C(20)
- Silt** (% of) St(20)
- Sand** (% of) Sd(20)
- Gravel** (% of) G(20)
- Sand** (Fine to Medium) Sd_{F-M}
- Gravel** (Fine to Coarse) G_{F-C SA-A}
- Subangular to angular**

OVERBURDEN

(Description uses BS 5930 and GSI guidelines)

BOULDER(S) (>200mm)

COBBLES (60 to 200mm)

GRAVEL (Homogeneous larger sized particles from 2 to 60 mm)

SAND (General, if without grain size description)
Particle sizes: 2 to 0.06mm. Three sub-categories distinguishable to the eye

Coarse **SAND** (2-0.6mm)

Medium **SAND** (0.6-0.2mm)

Fine **SAND** (0.2-0.06mm)



SILT (0.06 - 0.002mm)

CLAYS (<0.002mm)

CONCRETE

TARMACADAM

CRUSHED STONE or AGGREGATE

LANDFILL (eg plastic, glass, wood, domestic waste, concrete etc.)

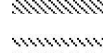
FILL OR BACKFILLED GROUND (unspecified)

COLLAPSED FORMATION (with possible voids) or **DRILL CHIPPINGS / MATERIAL RETURNED BY AIR FLUSH DRILLING**

LOSS (Blank - white)

TOP SOIL

PEAT (General) (with descriptions such as colour, plant remains evident, distinct H₂S smell etc) (H (Von Post) value associated commonly)



MONITORING POINT COMPLETIONS

- TS/C1/PH1** Terminal Site/Couple no./Phreatic no.
- PR/C2/P2** Peat Repository/Couple no./Piezometer no.
- H7** Von Post humification scale
- Push-on cap
- Screen
- Casing
- Porous tip
- Drive cone
- P1 PH1** Piezometer no. and Phreatic tube no.
- Bentonite pellets
- Cement-Bentonite grout
- Gravel pack, nominal 2-5mm in diameter
- Damp, wet and water strike respectively
- 1/2/03 Static water table (with date measured and hours since installation)

PLAN SKETCHES

- PWS1** Hand dug trial pits / Shallow pit excavations (JCB)
- TP1** Percussion Window Sampler (PWS) boreholes
- 100 BG** FID/PID in ppm Hydrocarbons with BG = background
- 99.791** Reduced levels - maOD Malin
- Oil pipeline
- Storage tanks (Overground and underground)

MONITORING POINT DESIGN FOR PEAT SUBSOILS

Push-on, female cap
The cap is loosely fitted to allow easy removal. The piezometer is labelled using indelible ink inside and outside the cap. A small hole is drilled in the side to enable air movement in and out of the piezometer.



Casing up-stand

The upstand is the height of the casing above ground level in meters. The height depends on local groundwater and surface water circumstances. The piezometer number is scrapped onto the side of the casing near the cap as with time the writing on the cap wears off. Upstands vary from 0.3 to 1.0m in height. The convention is allow a higher upstand for those piezometers positioned at a higher level.



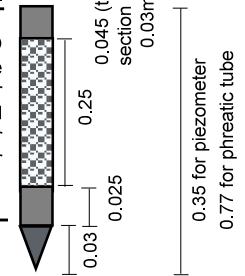
Ground level

Casing

The casing is black or dark grey coloured, flush-threaded, uPVC. The OD is 26.80mm and the ID is 18.40. The casing is flush-threaded to the piezometer tip.

Tube or Piezometer tip

This section is installed opposite the required formation. There are two sections to the piezometer tip. The inner tube section is 18.40mm ID, white in colour and involves extruded microporous polyethylene. The outer comprises grey or black coloured uPVC with 10 x 0.013m diameter holes per 0.10m of piezometer tip. Therefore the surface area exposed to the formation (peat) is small. The piezometer tube tip is flush-threaded, either male or female, to the piezometer casing. Threaded part is 0.03m long. The phreatic tube tip is longer than the piezometer tube tip to allow for greater water level fluctuations.



Drive cone

This is grey coloured, solid, uPVC, pushed or screwed into the tube or piezometer tip. No glue has been used. If the ground is soft, a push-in button cap may be used instead of a drive cone.

NOTES:-

The phreatic tubes are pushed by hand into the peat. The piezometers are pushed or driven into the peat and mineral soil after a narrow diameter hole has been formed using overburden drilling (Cobra or Percussion Window Sampler) / coring equipment (Gouge corer). The tubes and piezometers have three main functions: water table measurements, water sampling, permeability measurements.



Appendix E



Inchamore WF, Co. Cork

SI Trial Pit Photos







